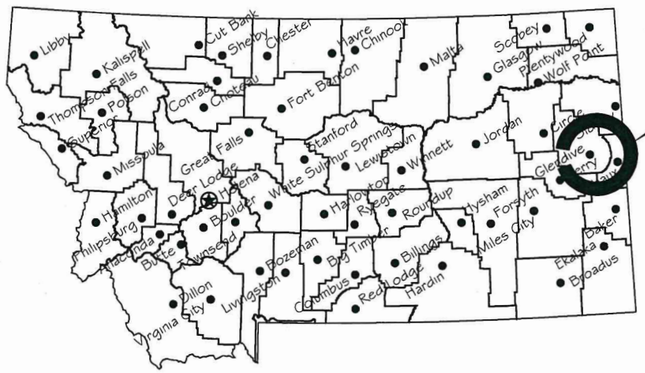
PROPOSED YURT INSTALLATION AT

MAKOSHIKA STATE PARK

DAWSON COUNTY , MONTANA

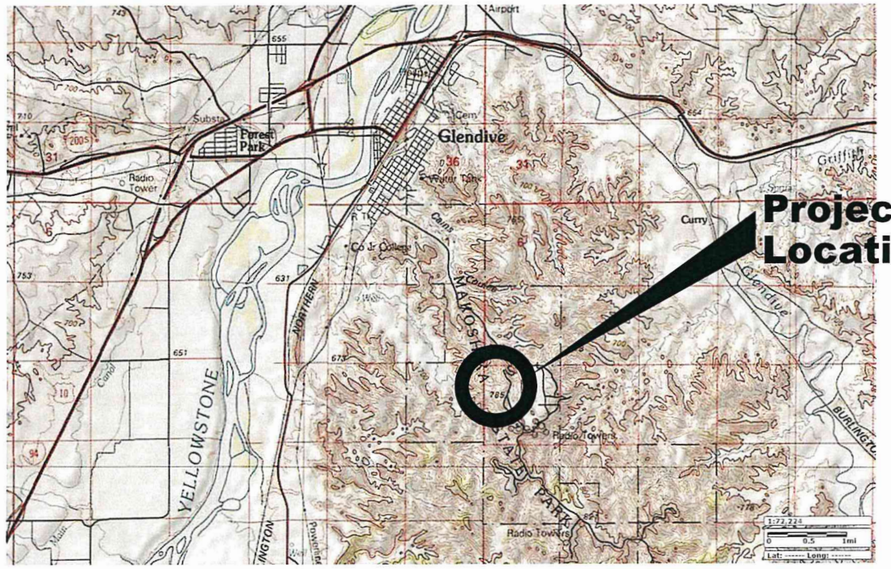
FWP PROJECT #7156104



Location Map

No Scale

North



Vicinity Map

No Scale

MONTANA FISH, WILDLIFE AND PARKS
DESIGN AND CONSTRUCTION

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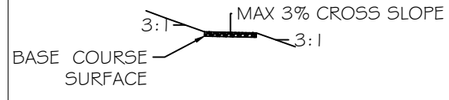
DRAWING INDEX

MONTANA
PHILLIP N. JAGODA
No. 49280PE
LICENSED PROFESSIONAL ENGINEER
3/16/21

B. Mangum 3/29/2021			MONTANA FISH, WILDLIFE & PARKS
DRAWN BY: DATE:	APPROVED BY: DATE:	APPROVED BY: DATE:	Cover Sheet Makoshika State Park
CHECKED BY: DATE:	APPROVED BY: DATE:	APPROVED BY: DATE:	SHEET: 1 of 12

WALKING PATH DETAIL



Notes:
1. Maintain max 3% cross-slope on path.
2. ¾" Minus or approved equal for base course s surface.
3. Salvage top soil to place on adjacent slopes.
4. Native dry land seed mix for revegetation to be approved by Park Manager.

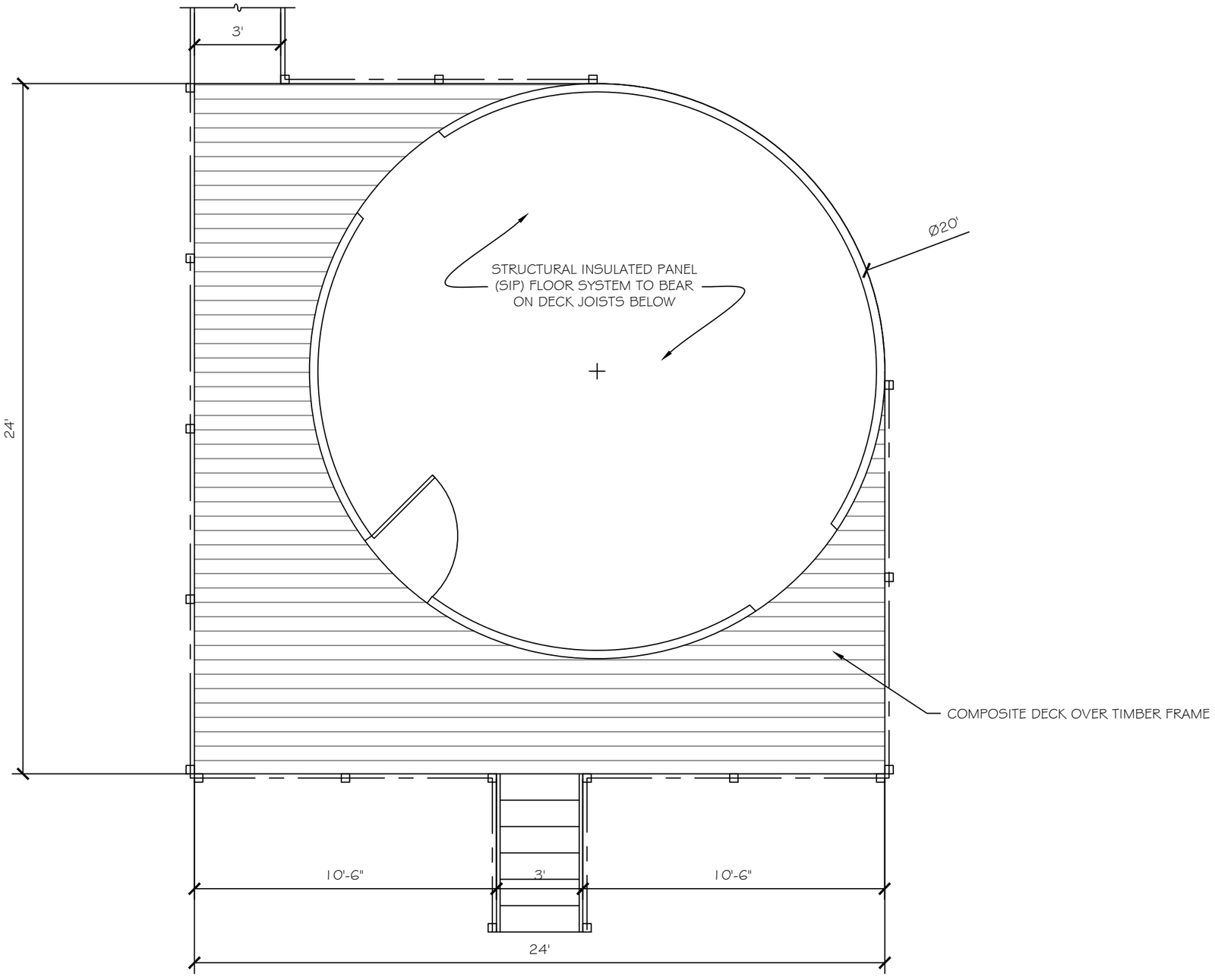
KH	6/30/21				
DRAWN BY:	DATE:	REVISED BY:	DATE:	APPROVED BY:	DATE:
?	?				
CHECKED BY:	DATE:	APPROVED BY:	DATE:	APPROVED BY:	DATE:

MONTANA FISH, WILDLIFE & PARKS

Site grading plan
Makoshika State Park

MONTANA STATE PARKS

SHEET: 2 of 12



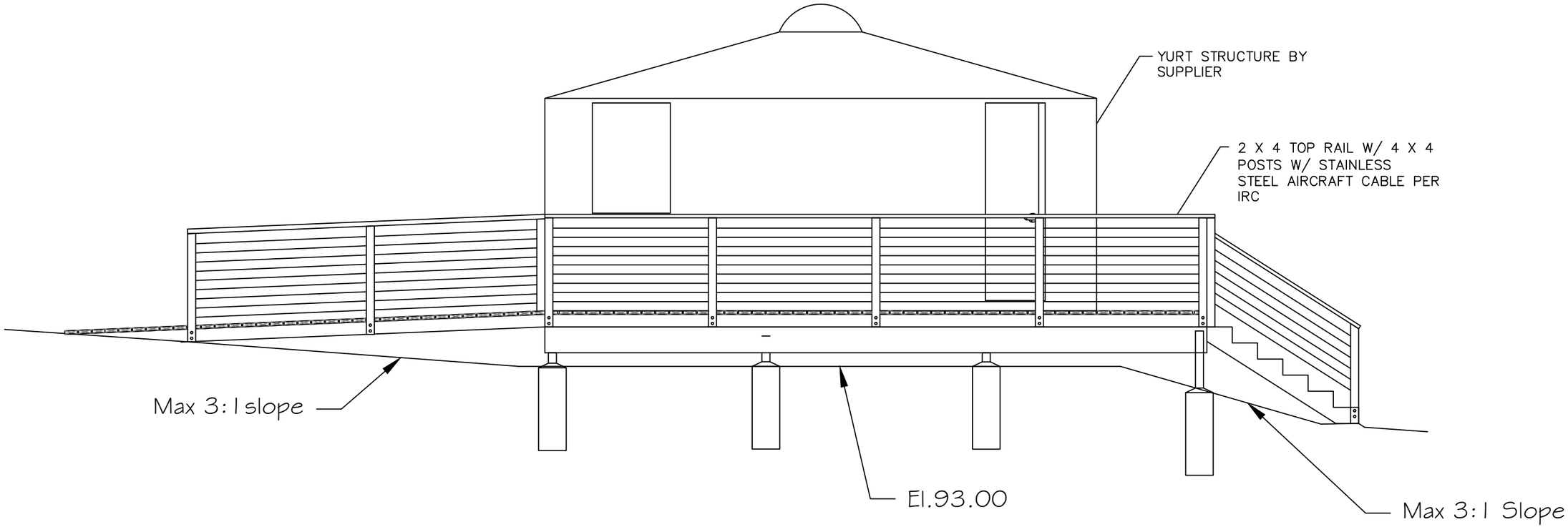
4' 3' 2' 1' 0 2' 4' 8'

Preliminary Yurt/Deck Plan

B. Mangum 3/29/2021 DRAWN BY: DATE:	APPROVED BY: DATE:	APPROVED BY: DATE:	MONTANA FISH, WILDLIFE & PARKS
CHECKED BY: DATE:	APPROVED BY: DATE:	APPROVED BY: DATE:	Yurt/Deck Plan Makoshika State Park

MONTANA STATE PARKS

SHEET: 3 of 12



YURT/DECKELEVATION

B.Mangum 3/29/2021 DRAWN BY: DATE: CHECKED BY: DATE:	REVISED BY: DATE: APPROVED BY: DATE:	APPROVED BY: DATE: APPROVED BY: DATE:	MONTANA FISH, WILDLIFE & PARKS	Yurt/Deck Plan Makoshika State Park	SHEET: 4 of 12

GENERAL NOTES
1. Contractor shall verify all dimensions and job site conditions before commencing work and shall report any discrepancies to the Engineer.
2. Contractor shall review and verify all dimensions shown on Structural drawings with those shown on Architectural drawings. Contractor shall notify the Architect of any discrepancies between the Architectural and Structural drawings and receive written clarifications of discrepancies before proceeding with construction.
3. Use written dimensions. Do not use scaled dimensions. Where no dimension is provided, consult the Engineer for clarification before proceeding with the work.
4. The Contractor is to review Architectural drawings for items that may not be shown on the Structural drawings. All openings in floors, roofs, or structural members that are not detailed per the Structural drawings must be reviewed by the Engineer before proceeding.
5. See Architectural drawings for non-load bearing elements. All non-loading bearing elements shall allow for vertical and lateral deflection of structural members.
6. The Contractor is responsible for implementing job site safety and construction procedures in accordance with national, state, and local safety requirements. The design, adequacy and safety of erection bracing, shoring, temporary supports, etcetera is the sole responsibility of the Contractor and has not been considered by the Engineer. The Contractor is responsible for the stability of the structure prior to the completion of all gravity and lateral framing, roof and floor diaphragms and finish materials.
7. The Contractor and sub-Contractor are responsible for the coordination of any penetration and/or use of structure for conduit, raceway, or the non-structural item with the Engineer prior to the installation of the non-structural item.
8. General notes shall not substitute for specifications. Conflicts between the two shall be brought to the Engineer's notice or the stricter criteria shall be used.
9. The Contractor will pay the Engineer for time and expense required to review, design, and coordinate items that were constructed not in conformance with these Construction Documents.
10. The Contractor is responsible for locating and the protection of all existing utilities and adjacent structures throughout all phases of construction.

INSPECTIONS
1. All construction shall be inspected by the building officials according to the above code.
2. It is recommended that the owner or the contractor hire other qualified licensed inspectors to provide inspection during construction.

DESIGN CRITERIA
1. CODE: International Building Code, 2018 Edition (IBC) and ASCE7-16.
2. DESIGN LOADS:
ROOF LOADS:
DEAD = 5 PSF
SNOW = 30 PSF ROOF
GROUND = 30 PSF

FLOOR LOADS:
DEAD = 15 PSF
LIVE = 40 PSF

LATERAL LOADS:
WIND - 111 MPH (3 Second Gust), Exposure C
SEISMIC - Site Class D, Risk Category II,
Sds = 0.077 Sd1= 0.044

3. ALLOWABLE SOIL BEARING PRESSURE = 1500 PSF

FOUNDATION
1. The foundation type and design criteria are based on assumed soil conditions and presumptive values from Section 1806 of the IBC. DCI suggests a professional geotechnical consultant should be hired by the Owner and/or Contractor to verify these assumptions.
2. The building is supported on spread footings bearing on competent sub-grade. The bottom of all exterior footings to bear 36" minimum below finished grade.
3. The bottom of all footings and slabs to bear on solid native, inorganic, undisturbed soil or approved compacted fill.
4. The Geotechnical Engineer shall perform an open excavation inspection prior to placing foundations to ensure the bearing capacity is satisfactory.
5. There shall be a minimum compaction to 95% of the maximum dry density (ASTM D698 Standard Proctor) of all backfill of soils under slabs on grade.
6. No concrete shall be placed in excavation containing water or on frozen soil.
7. In case conditions found at the site vary from those indicated on the drawings, the Architect is to be notified so that adjustments to the foundation can be made to meet actual field conditions.
8. No concrete shall be placed in footings or foundation wall without 48 hours notification to allow Engineer to observe the reinforcement if deemed necessary.
9. Backfill shall be placed against both sides of walls simultaneously. Contractor shall provide temporary shoring to prevent movement of walls if backfill is placed before the floor system is in place.
10. All footings shall be centered under walls and columns, unless noted otherwise.
11. The design and erection of all shoring, sheeting, soil stability, and dewatering is the sole responsibility of the Contractor. The Contractor shall hire a licensed Engineer to design all shoring and sheeting.
12. Utility and plumbing lines shall not go through or beneath the foundation, unless indicated otherwise.

CAST-IN-PLACE CONCRETE
1. Concrete properties shall be determined from designated Exposure Category F Class F2 as described in Section 4.2.1 of the latest edition of ACI 318, unless noted otherwise.
a. Minimum Compressive Strength: f'c = 4000 psi at 28 days, normal weight.
b. Maximum water/cement ratio limit (w/cm): 0.45
c. Air Content w/ 3/4" aggregate size = 6% +/- 1.5%
2. Maximum slump per Section 2.5.1 ACI 117: 3" +/- 1" for slabs and footings, 4" +/- 1" for walls, columns and beams.
3. Concrete shall be ready mixed in accordance w/ ASTM C94. Portland cement shall conform to ASTM C150, Type I or II. Normalweight aggregate shall conform to ASTM C33.
4. Curing compound shall conform to ASTM C309, Type 2, Class B.
5. Interior concrete slabs to receive a hard-troweled finish shall not utilize an air-entrained agent nor shall the air content exceed 3%.
6. All concrete shall have a minimum cementitious materials content of 470 pounds per cubic yard, unless noted otherwise. Minimum cementitious materials content for floors shall be 540 pounds per cubic yard, unless noted otherwise.
7. Concrete mixture shall include fly ash conforming to ASTM C618, Class C. Slabs: 15% minimum, 25% maximum by weight of cementitious material. All other uses: 25% minimum, 40% maximum by weight of cementitious material.
8. Calcium Chloride shall not be added to concrete.
9. Material, mixing, placement and workmanship shall be in accordance with the requirements of the latest edition of the "Building Code Requirements for Reinforced Concrete" (ACI 318) and Section 1905 of the IBC. Each proposed concrete mix shall include test data.
10. Concrete Placement: Cold weather is defined by ACI 306 as "The air temperature has fallen to, or is expected to fall below, 40°F"; when cold weather conditions exist, place concrete complying with ACI 306. Hot weather is defined by ACI 305 as "any combination of high air temperature, low relative humidity, and wind velocity tending to impair the quality of fresh or hardened concrete or otherwise resulting in abnormal properties"; when hot weather conditions exist, place concrete complying with ACI 305.
11. Testing of composite samples of fresh concrete obtained according to ASTM C 172 shall be performed on at least one composite sample for each 100 cubic yard or fraction thereof of each concrete mixture placed each day. Cast and laboratory and/or field cure at least two sets of two standard cylinder specimens for each composite sample according to ASTM C 31/C 31M. Test one set of two specimens at 7 days and one set of two specimens at 28 days according to ASTM C 39/C 39M. A compressive-strength test shall be the average compressive strength from a set of two specimens obtained from same composite sample and tested at age indicated. Strength of each concrete mixture will be satisfactory if every average of any three consecutive compressive-strength tests equals or exceeds specified compressive strength and no compressive-strength test value falls below specified compressive strength by more than 500 psi.
12. All Detailing, Fabrication, and Erection of reinforcing shall conform to latest edition of ACI "Manual of Standard Practice for Detailing Reinforced Concrete Structures" (ACI 315) and the current "Building Code Requirements for Reinforced Concrete" (ACI 318).
13. Reinforcing Steel:
ASTM A615 - Grade 40 for #3, Grade 60 for #4 and larger.
ASTM A706 - Where welding is required, at shear wall boundary elements (trim bars) and lateral frame elements
ASTM A185 - Welded Wire Reinforcement
14. The following minimum concrete cover shall be provided for reinforcement per ACI 318.
Concrete cast against and permanently exposed to earth: 3"
Concrete cast against forms and exposed to earth or weather
#6 through #18 bars : 2"
#5 bar and smaller: 1-1/2"
Concrete not exposed to weather or in contact with ground
Slabs, walls, joists: 3/4"
Beams, columns: 1-1/2"
15. Unless noted otherwise, lap splices in concrete shall be class "B" tension lap splices (2'-0" minimum) per the latest edition of ACI 318. Stagger alternate splices a minimum of one lap length. Lap welded wire fabric so that the overlap between outermost cross wires of each sheet is not less than the cross wire spacing plus 2 inches. All splice locations are subject to approval by Engineer and shall be made only where indicated on the drawings. Extend all horizontal reinforcing continuous around corners and intersections or provide bent corner bars to match and lap with horizontal bars at corners and intersections of footings and walls.
16. Provide bar supports and spacers to support all reinforcement in proper locations and wire adequately at intersections to hold bars firmly in position while concrete is placed. Bar supports and spacers which rest on or exposed surfaces shall be hot dipped galvanized or plastic coated.
17. Vertical dowels shall match the size and spacing of the wall reinforcement and be secured and supported in place prior to placing concrete, unless noted otherwise.
18. Welding of reinforcement is not permitted unless specifically noted or approved in writing by the Engineer.
19. All slabs on grade shall be placed directly on an approved vapor barrier placed over a 6" base of well-compacted gravel or stone.
20. Slab on grade shall be reinforced as indicated on plans. Where fiber reinforcing is indicated, a minimum of 1.5 pounds per cubic yard, or the manufacturer's stated minimum, shall be used.
21. Location of slab construction or pour joints must be approved by the Engineer if different from those shown on these drawings. Joints shall be placed at a maximum spacing of 15'-0" unless noted otherwise.
22. Isolation Joint Material shall be 1/2" thick full height of joint, unless noted otherwise.
23. All saw cut joints shall be "Sof Cut" sawn as soon as allowed by saw manufacturer recommendations. Joints shall be made within 4 hours in hot weather and within 12 hours in cold weathers after slab finish is completed.
24. Chamfer exposed edges of concrete beams and columns 3/4" unless noted otherwise.
25. Grout under all column base plates and beam bearing plates with non-shrink, non-metallic, 1" minimum thickness.
26. Electrical and mechanical conduits, raceways or other non-structural items shall not be placed on top of any floor deck, in an elevated or formed slab, or in a slab on grade without written consent from the Engineer.
27. Slab reinforcement shall extend through all construction joints, unless noted otherwise.
28. Slab penetrations shall not exceed 30% of the slab thickness and spaced no less than four diameters apart, unless specifically indicated otherwise.

WOOD
1. Framing lumber shall comply with the latest edition of the "National Design Specification" (NDS), American Forest & Paper Association / American Wood Council.
2. All sawn lumber shall be stamped with the grade work of a certified lumber grading agency. Moisture content shall not exceed 19%. All sawn lumber shall be Douglas Fir-Larch, unless noted otherwise.
3. Sawn Lumber:
Smaller dimension <4x nominal: no. 2 & better
Smaller dimension >4x nominal: no. 1 & better
4. Glu-Lams:
Continuous or cantilevered members - 24F-V8
Simple span members - 24F-V4
5. Wood Structural Panels: All panels shall conform to NER-108 and bear the stamp of the APA or an approved grading agency with the following span ratings:
Walls: 1/2" Nominal Thickness (15/32" Minimum) - 32/16, Sheathing
- Nail: 8d COMMON @ 6" o.c. - edges (uno)
8d COMMON @ 12" o.c. - field (uno)
Roof: 5/8" Nominal Thickness (19/32" Minimum) - 40/20, Sheathing
- Nail: 8d COMMON @ 6" o.c. - edges (uno)
8d COMMON @ 12" o.c. - field (uno)
Floor: 1 1/8" Nominal Thickness T&G Sturd-I-Floor or 48/24, T&G, Sheathing
- Glue & Nail: 12d COMMON @ 6" o.c. - edges (uno)
12d COMMON @ 10" o.c. - field (uno)
Provide blocking at wall panel edges and as designated on these drawings.
6. Framing Anchors: "Simpson" or approved equal. Install as per manufacturer's recommendations.
7. For nailing not shown on these drawings, use IBC nailing schedule, Table 2304.9.1.
8. Structural members shall not be cut for pipes, ducts, etc., unless specifically noted, detailed or approved in writing by the Engineer.
9. All exposed members or members in contact with concrete shall be preservative-treated wood stamped by an approved agency.
10. All steel, fasteners, and connectors in contact with wood that has ACQ formulation preservative treatment without ammonia shall be galvanized (G185) per ASTM A653 and ASTM A153 or Type 316L stainless steel. All steel, fasteners, and connectors in contact with wood that has ACQ formulation preservative treatment with ammonia shall be Type 316L stainless steel.
11. Wood stud walls shall be 2x6 at 16" o.c. unless noted otherwise on plans. Plate anchor rods shall be 5/8" diameter x 7" embedment with 3"x3"x1/4" plate washers placed not to exceed 4'-0" o.c. unless noted otherwise. A minimum of 2 threads shall extend above the nut, and the sill plate shall not be notched for the square washer and nut to be installed. Anchor rods shall be placed at all jambs, corners, intersections, and wall ends. All sill plates shall have a minimum of 2 anchor rods, and should have one anchor rod within 1'-0" of sill plate breaks or corners. All bottom plates or sills on concrete slabs on grade and on concrete or masonry foundations shall be preservative-treated wood stamped by an approved agency.
12. All non-bearing walls below framing shall be slip connected to allow for potential framing deflection and uplift.
13. Provide solid blocking between structural columns and framing below.

DESIGN SCOPE BY DCI ENGINEERS
1. Design shown on drawings by DCI include the following items.
a. Vertical and Lateral design for Yurt structure as shown
2. Design shown on DCI drawings does not include:
a. The platform, foundation, or floor system supporting the yurt.
b. The connection of the yurt to the platform, foundation, or floor system; including shear and hold-down connections.
c. Finishes, architectural items, windows, doors, moisture barriers, water proofing mechanical units, plumbing, or electrical items. For the design and/or details of any other items not mentioned in #1 above, see additional drawings developed by others.
c. Structural details provided for this project are typical details intended to illustrate general conditions and not to specifically address every minor variation. They are not necessarily drawn to scale. The contractor/builder shall use details as a guide in construction and fabrication, while verifying specific dimensions/angles and other conditions that may exist (refer to S1/A item NO.4). If the owner/contractor/builder requires details to be drawn to scale, dimensioned and specific to a given situation, please contact DCI and we can arrange to provide you with such details per the Additional Services fee schedule.

ABBREVIATIONS

AB	ANCHOR BOLT/ROD	PSF	POUNDS PER SQUARE FOOT
APA	AMERICAN PLYWOOD ASSOCIATION	PSI	POUNDS PER SQUARE INCH
ARCH	ARCHITECT (URAL)	R	RADIUS/RISE (R)
BLKG	BLOCKING	REF	REFERENCE
BM	BEAM	REINF	REINFORCE (D), (ING)
BOT	BOTTOM	REQD	REQUIRED
BRG	BEARING	SECT	SECTION
BU	BUILT-UP	SHT	SHEET
CL ℄	CENTER LINE	SHTHG	SHEATHING
COL	COLUMN	SIM	SIMILAR
CONC	CONCRETE	SQ	SQUARE
CONN	CONNECT (ION)	STL	STEEL
CONT	CONTINUOUS, CONTINUE	STRUCT	STRUCTURAL
DIA Ø	DIAMETER	SUB FL	SUBFLOOR
DIM	DIMENSION	SW	SHEAR WALL
DWG	DRAWING	T&B	TOP AND BOTTOM
(E)	EXISTING	T&G	TONGUE AND GROOVE
EL	ELEVATION	TBD	TO BE DETERMINED
FND	FOUNDATION	THRU	THROUGH
FTG	FOOTING	TO	TOP OF
GL	GLUE LAMINATED (BEAM)	TOB	TOP OF BEAM
GYP	GYPSUM	TOC	TOP OF CONCRETE
HDR	HEADER	TOF	TOP OF FOOTING
K	THOUSAND (KIP)	TOM	TOP OF MASONRY
KIP	THOUSAND POUND	TOP	TOP OF PIER
KSI	KIPS PER SQUARE INCH	TO PAR	TOP OF PARAPET
LL	LIVE LOAD	TOS	TOP OF STEEL
MFR	MANUFACTURER	TO SHTHG	TOP OF SHEATHING
(N)	NEW	TO SUB FL	TOP OF SUBFLOOR
NTS	NOT TO SCALE	TOW	TOP OF WALL
OC	ON CENTER	TYP	TYPICAL
PERP	PERPENDICULAR	UNO	UNLESS NOTED OTHERWISE
PL ℄	PLATE	(V)	VERIFY
PLYWD	PLYWOOD	WD	WOOD
PREFAB	PREFABRICATE	WWF	WELDED WIRE FABRIC

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REGISTERED PROFESSIONAL ENGINEER

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PROJECT IDEN.
20'-0" Yurt & Platform
Mikoshika State Park
GLENDIVE, MT

ISSUE BLOCK

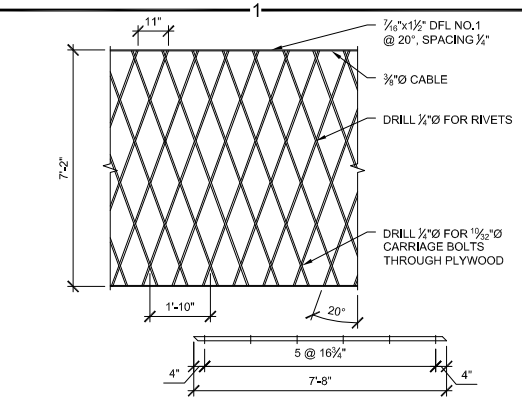
-	3/26/2021 ISSUE

MANAGEMENT

PROJECT NO.:	21141-0076
DRAWN BY:	TRM
CHECKED BY:	MKN

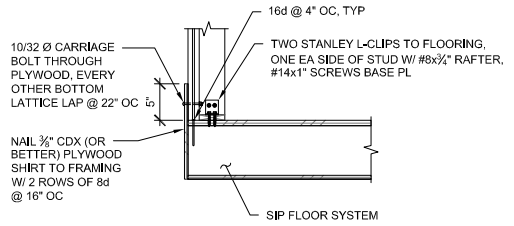
SHEET TITLE
STRUCTURAL NOTES

SHEET IDENTIFICATION
S0.1
5 OF 12



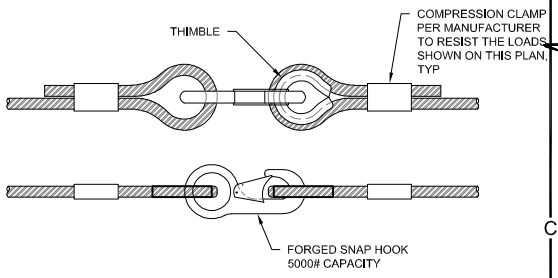
1 WALL LATTICE

SCALE: ⅜"=1'-0"



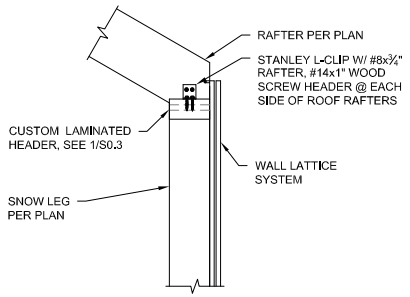
2 HOLDOWN

SCALE: 1"=1'-0"



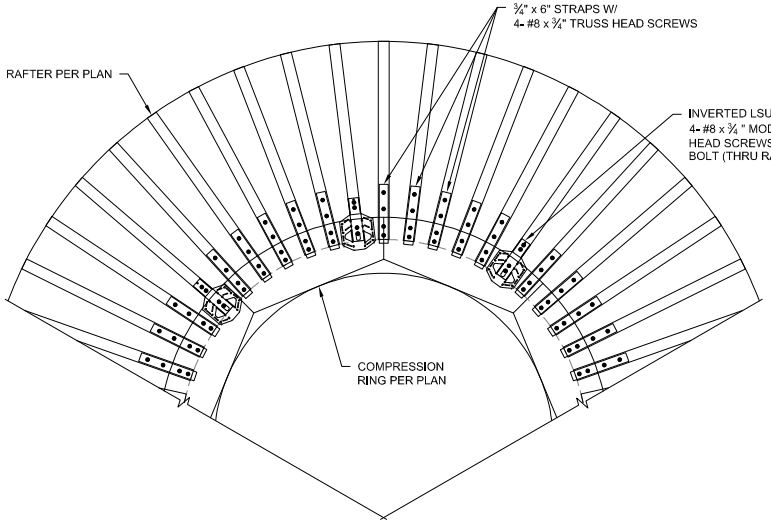
3 CABLE END DETAIL

SCALE: 1"=1'-0"



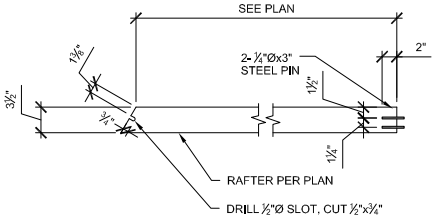
4 HEADER AT DOOR

SCALE: 1"=1'-0"



6 RAFTER TO COMPRESSION RING CONNECTION

SCALE: 1"=1'-0"



5 RAFTER CUT DETAIL

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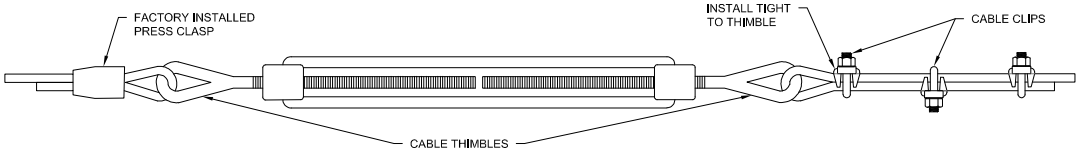

CLIP SIZE (IN INCHES)	ROPE SIZE (IN INCHES)	TURNBUCKLE SIZE (IN INCHES)	MINIMUM NUMBER OF CLIPS	MINIMUM AMOUNT OF ROPE TO TURN BACK (IN INCHES)	TORQUE FOOT LBS
¼"	¼"	5/16"	3"	10½" (4¾" BETWEEN CLIPS)	15
5/16"	5/16"	⅜"	3"	11½" (5¼" BETWEEN CLIPS)	30
⅜"	⅜"	½"	3"	14" (6½" BETWEEN CLIPS)	45
MULTIPLE CABLE TURNBUCKLES SHOULD BE SPACED EVENLY APART FROM EACH OTHER THROUGHOUT THE YURT CIRCUMFERENCE					

ALL U-BOLTS OVER HEAD END OF THE ROPE, SADDLE ON LIVE END. TORQUE TO SPECS IN TABLE BELOW FOR APPROPRIATE ROPE SIZE.

7 INNER CABLE END DETAIL

SCALE: 1"=1'-0"

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REGISTERED PROFESSIONAL ENGINEER
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CIVIL / STRUCTURAL

PROJECT IDEN.

20'-0" Yurt & Platform
Mikoshika State Park
GLENDIVE, MT

ISSUE BLOCK

-	3/26/2021 ISSUE

MANAGEMENT	
PROJECT NO.:	21141-0076
DRAWN BY:	TRM
CHECKED BY:	MKN

SHEET TITLE

DETAILS

SHEET IDENTIFICATION

S0.2

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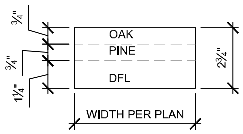
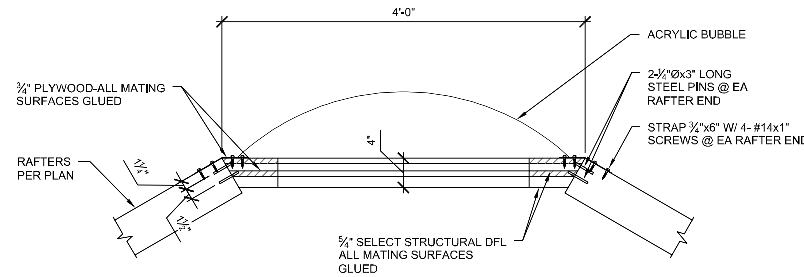
115



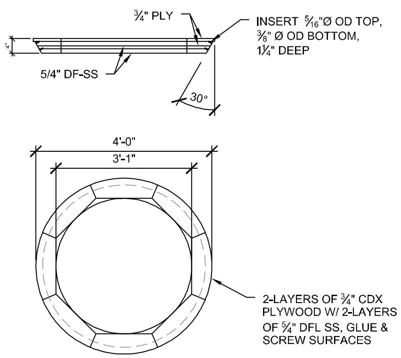
1 HEAD - LAMINATE SCALE

SCALE: 3"=1'-0"



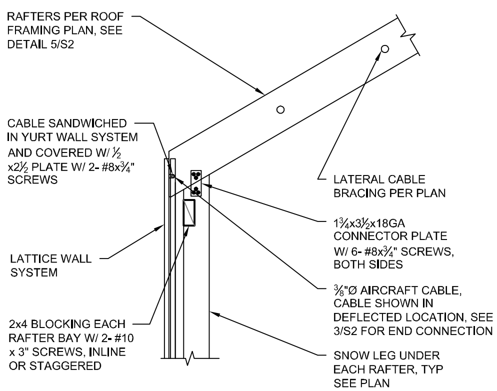
2 COMPRESSION RING - SECTION

SCALE: 1"=1'-0"



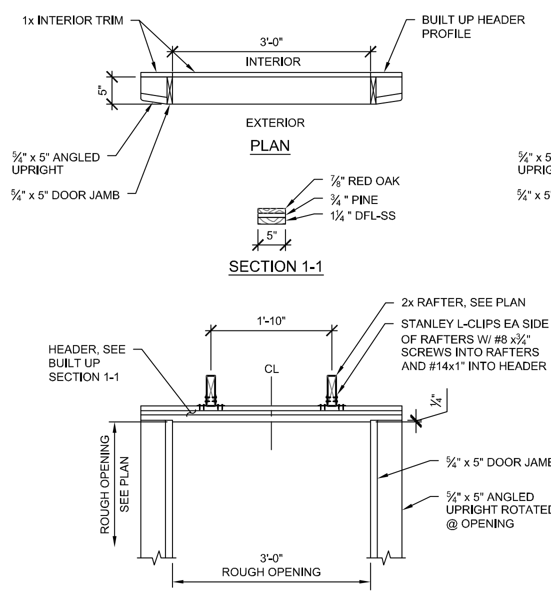
3 COMPRESSION HUB

SCALE: 1"=1'-0"



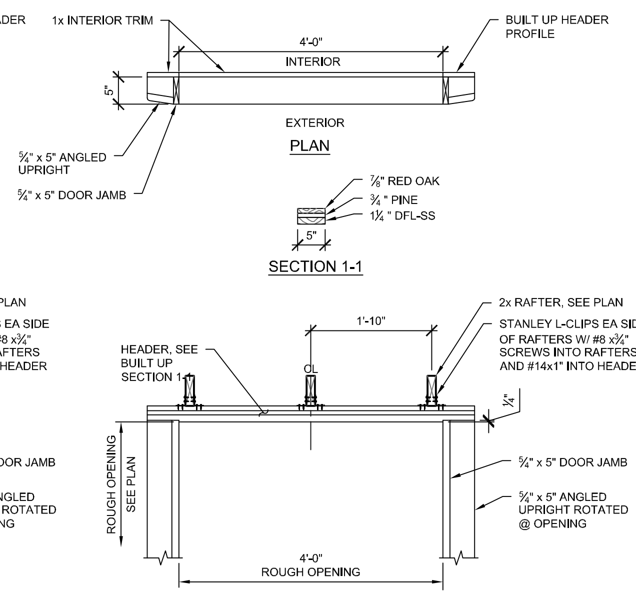
4 EAVE DETAIL

SCALE: 1"=1'-0"



5 TYP 3'-0" WINDOW & DOOR HEADER

SCALE: ¾"=1'-0"



6 TYP 4'-0" FRECH DOOR HEADER

SCALE: ¾"=1'-0"

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DETAILS

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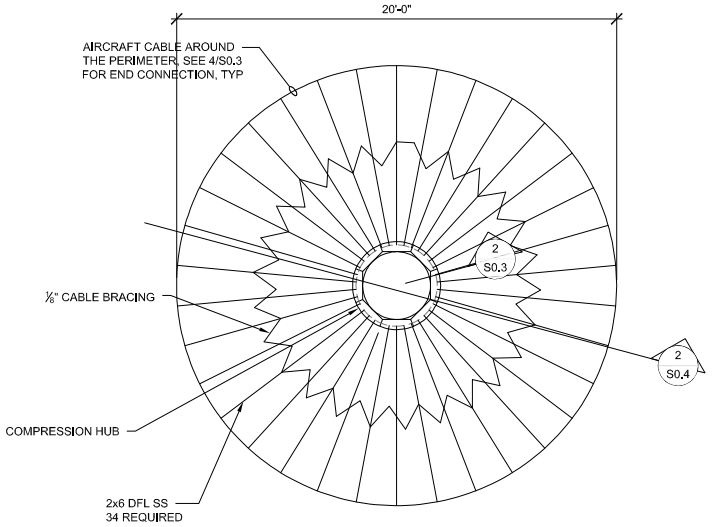
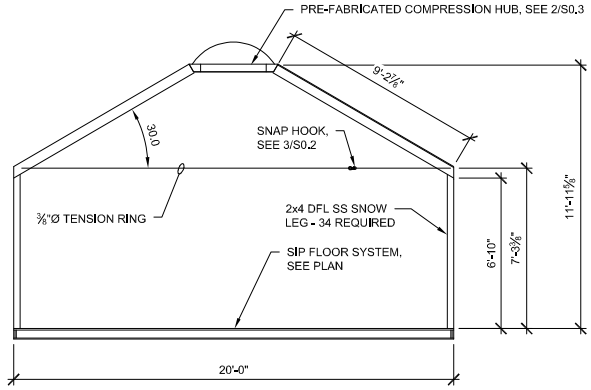


1 ROOF FRAMING PLAN FOR 20' YURT

SCALE: ¼"=1'-0"

0' 2' 4' 8'



2 YURT SECTION

SCALE: ¼"=1'-0"

0' 2' 4' 8'

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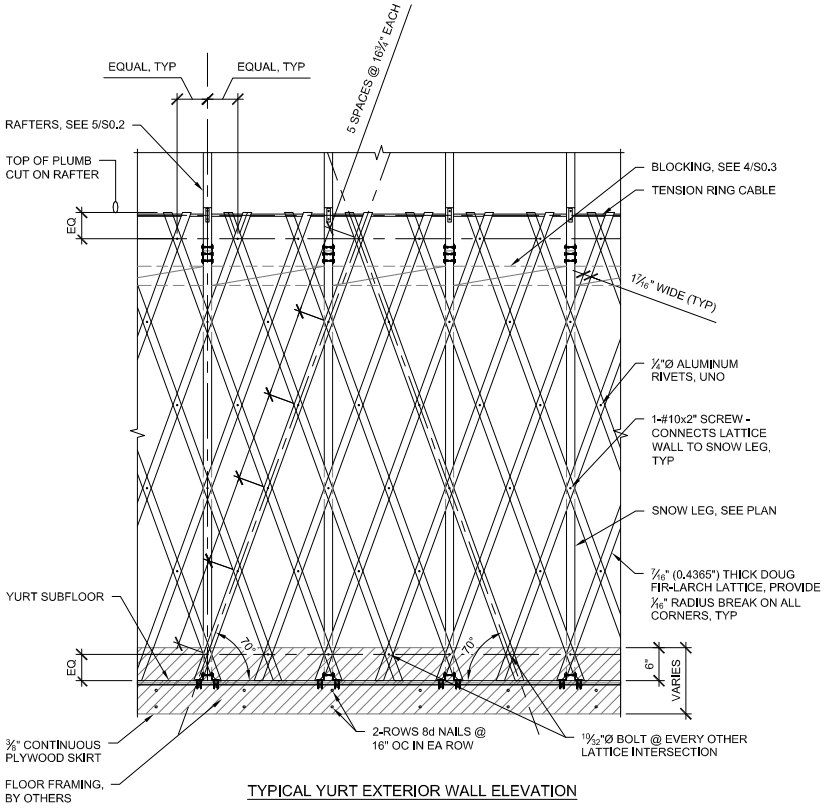
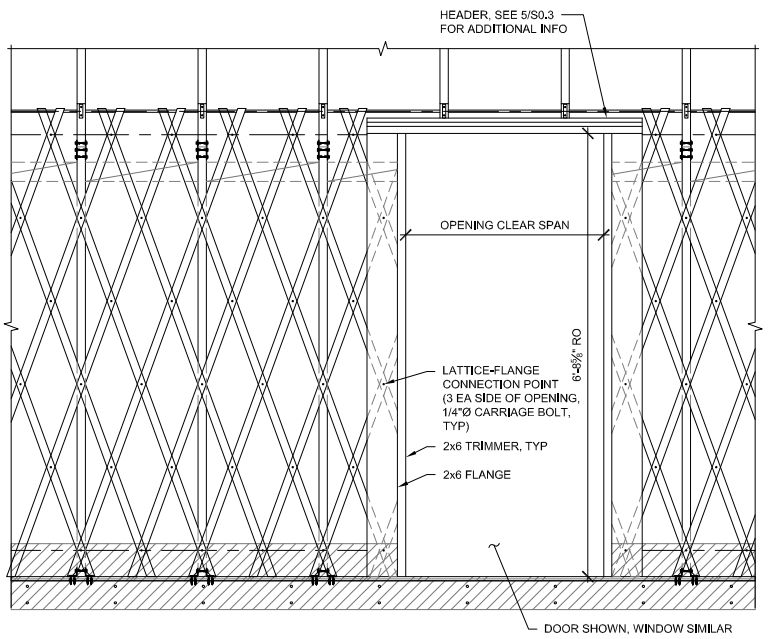


TYPICAL YURT EXTERIOR WALL ELEVATION

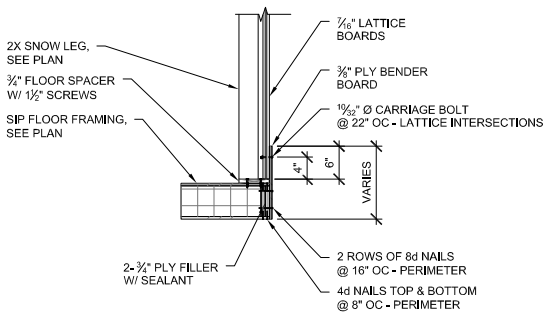


TYPICAL YURT EXTERIOR WALL ELEVATION AT OPENINGS

1 YURT EXTERIOR LATTICE WALL ELEVATION

SCALE: ¼"=1'-0"

0' 2' 4' 8'



2 FLOOR EDGE DETAIL

SCALE: ¼"=1'-0"

0' 2' 4' 8'

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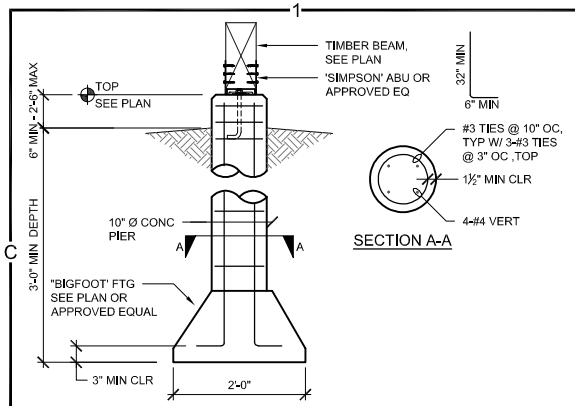
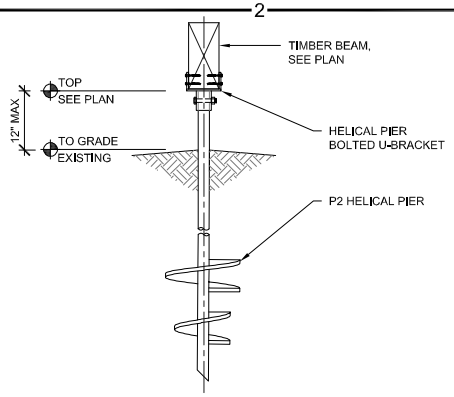


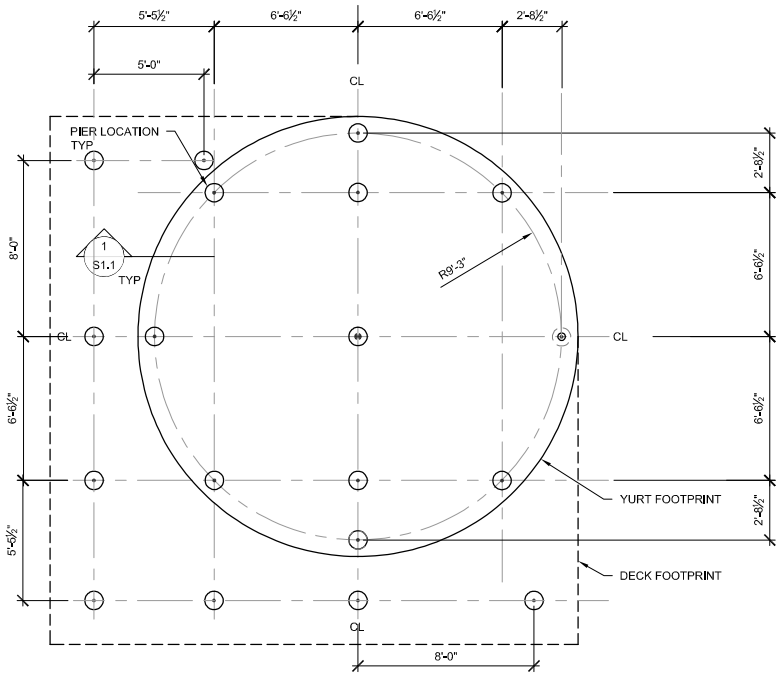
1 CONC PIER WITH SONO TUBE

SCALE: NTS



2 ALT - HELICAL PIER

SCALE: NTS



- FOUNDATION PLAN

SCALE: ¼"=1'-0"

0' 2' 4' 8'

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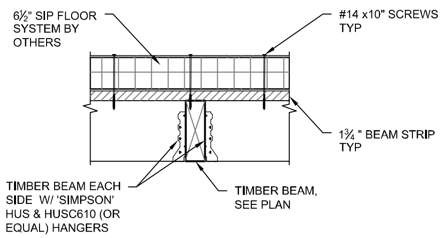
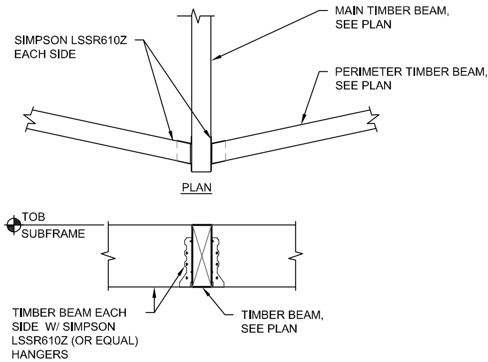


1 TYP BEAM CONN. @ INTERIOR BEAM

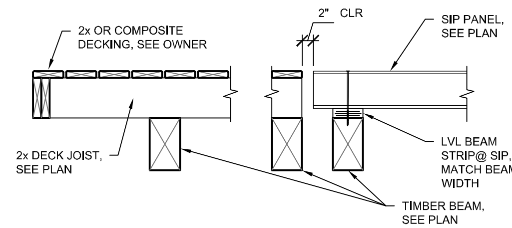
SCALE: ¾"=1'-0"



2 TYP PERIMETER BEAM CONN. @ GL BEAM

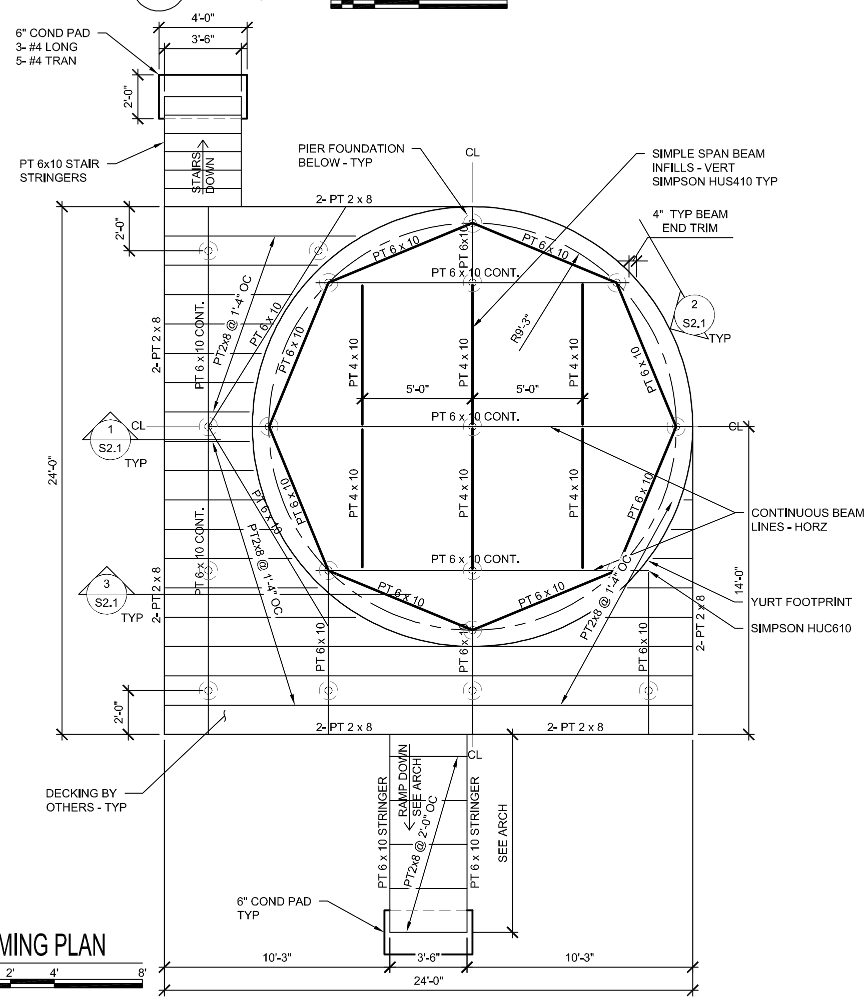
SCALE: ¾"=1'-0"

ALL TIMBER BEAMS TO BE PRESSURE TREATED FOR NEAR GROUND CONTACT



3 TYP BEAM CONN. @ 2x DECK & SIP PLATFORM

SCALE: ¾"=1'-0"



- MAIN PLATFORM & DECK FRAMING PLAN

SCALE: ¼"=1'-0"

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MAIN PLATFORM BEAM PLAN

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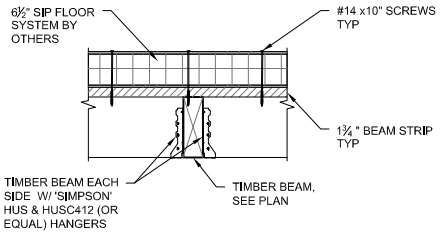
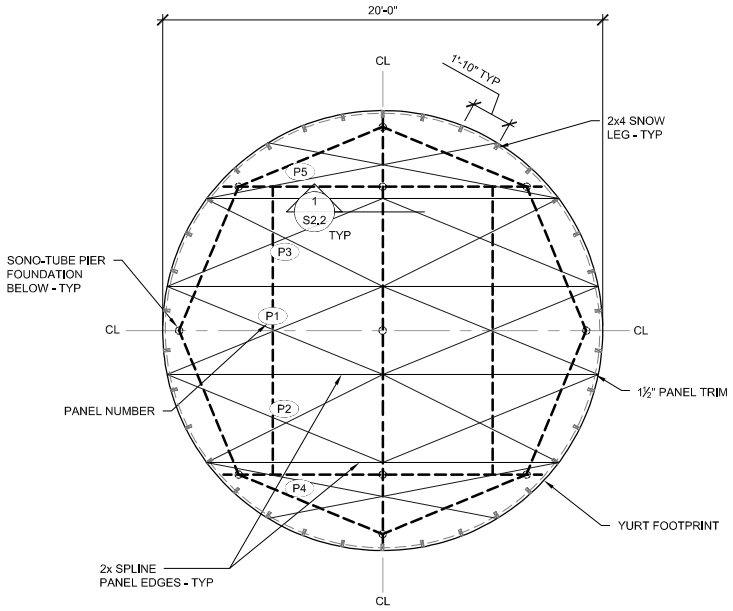


1 TYP BEAM CONN. @ INTERIOR BEAM

SCALE: ¾"=1'-0" 0" 8" 16" 32"

P1 6½" SIP FLOOR PANELS - TYP



- MAIN PLATFORM PANEL LAYOUT

SCALE: ¼"=1'-0" 0' 2' 4' 8'

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PLATFORM SIP PANEL LAYOUT

SHEET IDENTIFICATION

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