

# Changes in Groundwater Chemistry and Groundwater Elevation in Response to Peak Hydrograph Conditions



*Submitted to:*

## **Geomorphology Subcommittee**

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Environmental Solutions  
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*Date:*

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# Table of Contents

Section	Page
1.0 Introduction .....	1.
1.1 Background.....	1.
1.2 Approach.....	1.
2.0 Hydrogeologic Conceptual Model.....	6
3.0 Methods .....	8.
3.1 Hydraulic Conductivity .....	8.
3.2 Groundwater Chemistry.....	8.
4.0 Results and Discussion .....	12
4.1 Hydrostratigraphic Units .....	12
4.1.1 Control Microwatershed.....	12
4.1.2 Treated Microwatershed.....	12
4.2 Hydraulic Characteristics .....	13
4.2.1 Control Microwatershed.....	13
4.2.2 Treated Microwatershed.....	13
4.3 Groundwater Flow and Direction.....	14
4.3.1 Control Microwatershed.....	14
4.3.2 Treated Microwatershed.....	14
4.4 Groundwater chemistry Characterization.....	16
5.0 Conclusion .....	20
6.0 References .....	21

## List of Figures

Figure	Page
Figure 1.1. Location map indicating position of control and treated microwatersheds.....	3
Figure 1.2. Detailed map of Governor's Demonstration Project control site.....	4
Figure 1.3. Detailed map of Governor's Demonstration Project treated site.....	5
Figure 2.1. Conceptual model of the three hydrologic/soil units.....	7
Figure 2.2. Typical cross section through the Clark Fork River floodplain showing variable materials.....	10
Figure 2.3. Expected changes in groundwater chemistry during variation in river stage.....	11
Figure 4.1. Groundwater elevation at the Governor's Demonstration Project treated microwatershed and river stage at the Clark Fork River near Galen USGS gauging station.....	15
Figure 4.2. Groundwater elevation at the Governor's Demonstration Project control microwatershed and river stage at the Clark Fork River near Galen USGS gauging station.....	16
Figure 4.3. Arsenic concentrations at Governor's Demonstration Project treated and control microwatersheds for the period of record.....	17
Figure 4.4. Copper concentrations at Governor's Demonstration Project treated and control microwatersheds for the period of record.....	17
Figure 4.5. Zinc concentrations at Governor's Demonstration Project treated and control microwatersheds for the period of record.....	18

# List of Tables

Table	Page
Table 4.1. Concentrations of constituents of concern for monitor wells MW-1A, MW-1B, MW-2A, and MW-2B.....	19

## 1.0 Introduction

### 1.1 Background

Tailings material deposited in some parts of the Clark Fork River floodplain can become saturated when groundwater elevations increase as river stage increases, providing a potential load source of constituents of concern to the Clark Fork River. The changes in shallow groundwater chemistry and the shallow groundwater gradient and flow direction near the Clark Fork River must be quantified to determine if there is any load of constituents of concern (arsenic, cadmium, copper, lead, and zinc) associated with peak stage and high groundwater conditions during spring runoff and snowmelt processes. Several monitor wells were installed within two small watersheds along the Clark Fork River floodplain as part of the Governor's Demonstration Project Monitoring (Schafer & Associates, 1997). This report examines shallow groundwater flux and shallow groundwater chemistry at the control and treated microwatersheds during peak river stage and shallow groundwater elevations, and how they change when river stage decreases to base flow levels.

### 1.2 Approach

The hydraulic properties of the soil, tailings, and aquifer materials were examined along with changes in shallow groundwater chemistry. Hydraulic properties of alluvium were determined using pumping tests, slug tests, and bore hole permeameter tests. Groundwater samples were collected from shallow groundwater monitoring wells and drive point piezometers completed at the Governor's Demonstration Project control and treated microwatersheds, located in the river floodplain, during base flow and during the climbing limb, peak, and declining limb portions of the river hydrograph.

The seasonal variation in groundwater chemistry and elevation was examined at the Governor's Demonstration Project control and treated microwatersheds, depicted on Figure 1.1. The treated microwatershed was reclaimed by incorporating lime using deep plow methods and revegetating. A detailed description of the reclamation work can be found in the Final Report on the Clark Fork River Governor's Demonstration Project (1991). The control microwatershed has not been reclaimed. Both watersheds have been instrumented to observe climatic conditions, measure surface water run off, and measure changes in volumetric water content in the near surface soil. Monitor wells and drive point piezometers have been installed to measure groundwater elevation and to collect groundwater samples.

A large quantity of groundwater chemistry and hydrologic data were available at the Governor's Demonstration Project control and treated microwatersheds as a result of aquifer testing and extensive water quality sampling. Therefore, the analysis of potential changes in shallow groundwater chemistry and groundwater elevation in response to spring peak hydrograph conditions was conducted at the two microwatersheds. Detailed maps of the two microwatersheds, displaying monitor well,

observation well, and drive point piezometer locations, are presented in Figures 1.2 and 1.3.

The spring peak hydrograph is associated with water entering the Clark Fork River from annual snowmelt processes and from precipitation during snow melt. Changes in river stage and groundwater elevation associated from a single precipitation event are not analyzed in this report.

Prepared for **ARCO**

**FIGURE 1.1**

DATE: December 15, 1997  
SCALE: 1:12,000

FILE: GOVDEMO.APR  
PLOT: GOVDEMO.PRT

**Springs**

**Warm Springs  
Ponds**

**Governor's  
Demonstration  
Project**

**Perkins Lane**



**Figure 1.1 Location of Clark Fork River Governor's Demonstration Project.**



Scale: 1" = 50'  
Contour Interval 0.2'



Scale in Feet

### LEGEND

- SURFACE WATER FLUME AND SAMPLING LOCATION
- GROUNDWATER MONITORING WELL
- LYNSMETER
- NEUTRON PROBE
- WEATHER STATION
- DRIVE POINT
- MICRO-WATERSHED BOUNDARY  
38,000 SQ. FT. (894 ACRES)
- FENCE LINE
- EDGE OF WATER LINE

## ARCO - CLARK FORK RIVER GOVERNOR'S DEMONSTRATION PROJECT MONITORING STUDY

### UNTREATED WATERSHED #2 (CONTROL)

1.2

	DATE: DECEMBER 15, 1997	SCALE: 1" = 50'	PROJECT NO. 147-22
	NOTES:	CONTOUR INTERVAL: 0.2'	CADD: I. THATCHER
ENVIRONMENTAL SOLUTIONS FOR WATER, WASTE AND LAND SCHAFFER & ASSOCIATES, INC. 865 TECHNOLOGY BLVD. BOZEMAN		FILENAME: UNTREAT3.DWG FIGURE:	

1142300 E  
1142400 E  
1142500 E

RIGHT SIDE  
EDGE OF WATER  
CLARK FORK RIVER  
732500 N

732500 N

732400 N

732300 N





Scale: 1" = 50'  
Contour Interval 0.2'



Scale in Feet

### LEGEND

- SURFACE WATER FLUME AND SAMPLING LOCATION
- GROUNDWATER MONITORING WELL
- LYSIMETER
- NEUTRON PROBE
- WEATHER STATION

MICRO-WATERSHED BOUNDARY  
20,717.77 SQ. FT. (4.75 ACRES)

- FENCE LINE
- EDGE OF WATER LINE

CLARK FORK RIVER

LYSIMETER  
SM18-20

NEUTRON  
PROBE-1A

LYSIMETER  
SM18-14

GROUNDWATER  
MONITORING WELL-1A

LYSIMETER  
SM18-38

SURFACE WATER  
FLUME

LYSIMETER  
SM18-16

GROUNDWATER  
MONITORING WELL-1R

LYSIMETER  
SM18-40

LYSIMETER  
SM18-25

NEUTRON  
PROBE-1B

DP-1A

DP-1B

## ARCO - CLARK FORK RIVER GOVERNOR'S DEMONSTRATION PROJECT MONITORING STUDY

TREATED WATERSHED #1

1.3



DATE:	DECEMBER 15, 1987	SCALE:	1" = 50'	PROJECT NO.	147-22
NOTES:		CONTOUR INTERVAL:	0.2'	CADD:	T. HATCHER
				FILENAME:	TREATED.DWG

ENVIRONMENTAL SOLUTIONS FOR WATER, WASTE AND LAND  
SCHAEFER & ASSOCIATES, INC. 985 TECHNOLOGY BLVD. BOZEMAN, MONTANA

1141900PFE

1141800PFE

1141700PFE

729500PFE

729400PFE

729300PFE

## 2.0 Hydrogeologic Conceptual Model

The majority of the tailings mapped in the floodplain (Schafer and Associates, 1988) are overbank deposits. A majority of these deposits formed during previous flood events and are underlain by "point bar" deposits. Along the Clark Fork River the upper point bar deposits have had one or more episodes of tailings deposits representing historic flooding (Nimmick, 1990). The topographic position of most floodplain tailings deposits is in a well-drained location at least 1 to 4 feet above the base flow groundwater elevation (Schafer and Associates, 1997).

Drill log and soil pit characterization indicated that the shallow groundwater system could be separated into three layers (Figure 2.1) with distinct hydrologic and soil properties. The top layer (layer 1) is generally tailings material, soil or a combination of both and contains a large quantity of fine material. A sand and gravel material with fines dominates the middle layer (layer 2). Layer 3 consists of clean heaving sand or a cobble and gravel mix. The three-layer hydrologic/soil model appeared to be pervasive in the shallow groundwater system in the Clark Fork River floodplain.

The hydrologic boundaries of the shallow groundwater system, along with the physical arrangement of layers, determine the groundwater flow rate and direction (Figure 2.2). The three-layer shallow groundwater system is connected to the Clark Fork River channel in a downgradient direction. The shallow groundwater system is recharged laterally by regional groundwater at the edge of the package of alluvial deposits paralleling the Clark Fork River. Finally, a small amount of groundwater recharge may occur seasonally when water flows through the vadose zone. Changes in the elevation of the Clark Fork River, and the rate of lateral and vertical recharge will determine the shallow groundwater levels.

Seasonal changes in groundwater level will be caused by variations in river stage. Because shallow groundwater and tailings in the variably saturated zone contain higher metal concentrations than the permeable gravel layer, these changes in groundwater elevation may induce changes in metal flux through the groundwater pathway. The hydrochemical model for the Clark Fork alluvial groundwater system is determined by the layer sequence within the shallow groundwater system, differences in hydraulic conductivity, seasonal variation in head, and variable chemistry between layers. As a consequence of the hydrochemical model, the expected pattern of change in metal flux due to variation in river stage can be predicted (Figure 2.3). If seasonal variation in the hydrograph increases metal flux to the river, the highest metal concentrations should be observed in layer 3 (gravels) during the falling limb of the hydrograph.

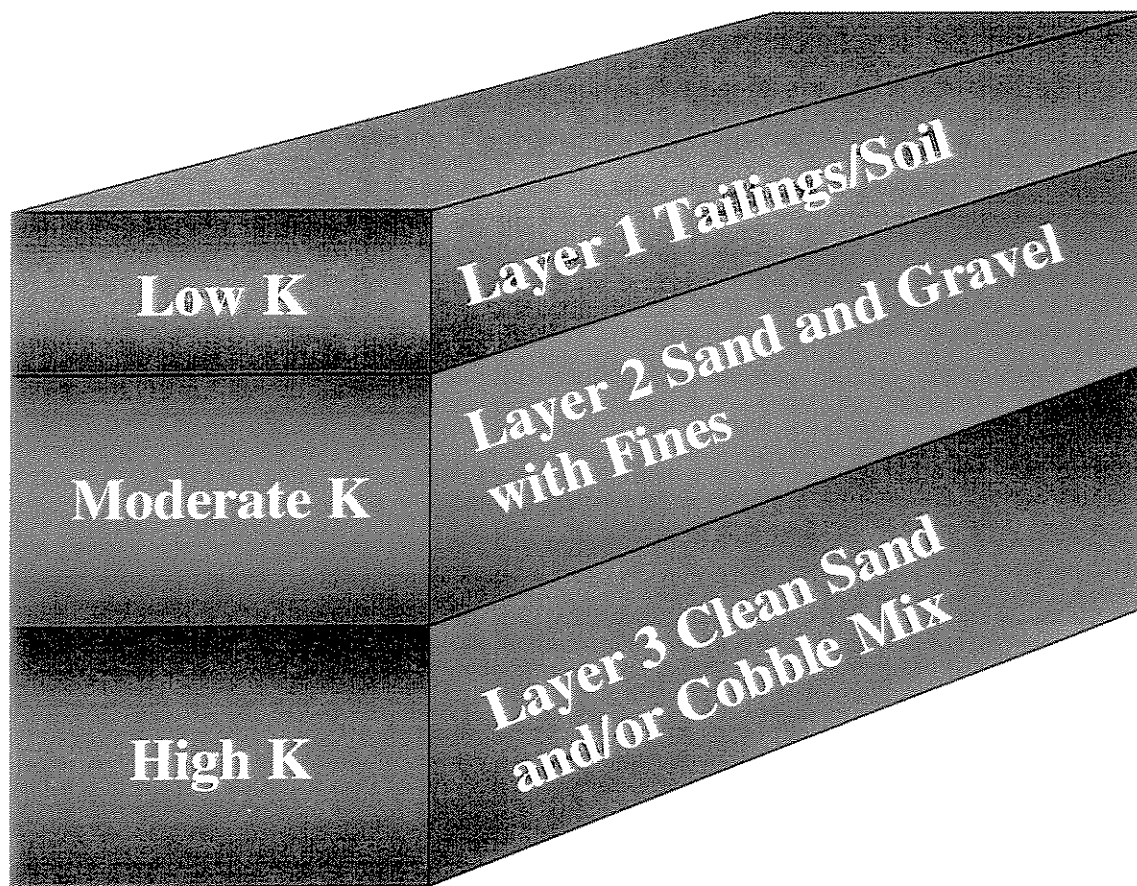


Figure 2.1. Conceptual model of the three hydrologic/soil units.

## 3.0 Methods

Aquifer characteristics were determined using a variety of methods, including pumping tests, slug tests, and bore hole permeameter tests. The hydraulic gradient and flow direction was determined by measuring water elevation in monitor wells and drive point piezometers. Samples were collected from monitor wells and drive point piezometers several times during the year to determine seasonal changes in shallow groundwater chemistry.

### 3.1 Hydraulic Conductivity

Pumping tests and slug tests were used to determine the hydraulic characteristics of conceptual model layers 2 and 3 and bore hole permeameter tests were used to determine the near surface hydraulic properties, layer 1. Results of the tests were used to characterize groundwater movement in the control and treated sites.

A 48-hour constant-rate pumping test was completed at the control site during the summer of 1996. The aquifer test was conducted with one pumping well, seventeen observation wells, and five drive point piezometers. A detailed description of the pumping test and analysis is provided by Schafer and Associates (1997a).

Slug tests were performed on five observation wells at the control site (OBS-1B, OBS-2B, OBS-3B, OBS-4B, and OBS-5) and 2 monitoring wells at the treated site (MW-1A and MW-1B). Slug tests were completed and analyzed as described by Schafer and Associates (1997b).

A bore hole permeameter was used to measure in situ saturated hydraulic conductivity of the unsaturated tailings material (control site) and amended tailings material (treated site) overlying the shallow alluvial aquifer material. Measurements were completed at 0.6 and 1.3 feet at the control site and at 0.3, 1.0, and 1.7 feet at the treated site. Gradient and Direction of Groundwater Flow

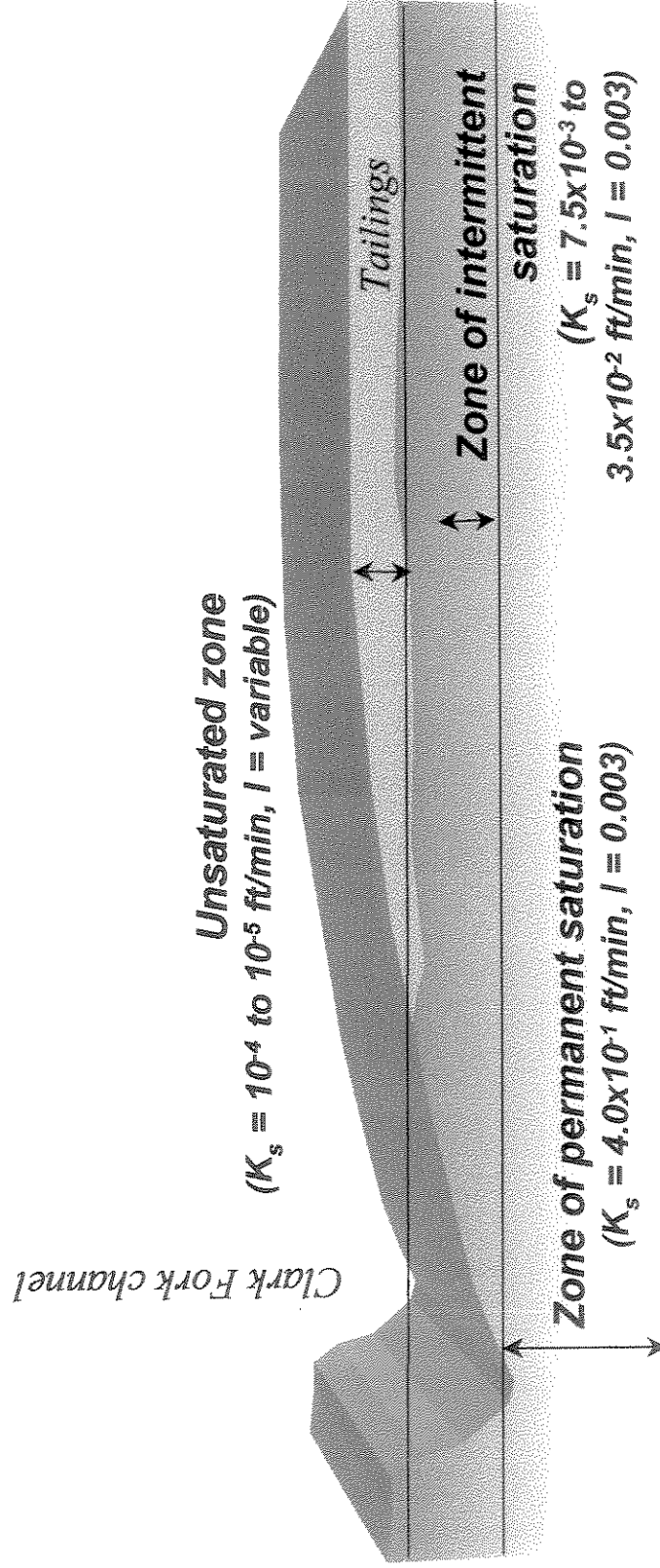
Static water levels were measured in each monitor well, observation well, and drive point piezometer using an electronic water level probe. Measurements were made to the nearest 0.01 feet. A level-line survey was conducted at each site prior to static water level measurements. Combining the level-line survey with the static water level measurements allowed the calculation of actual groundwater elevations in each well and drive point. A staff gauge installed at each site allowed for direct comparison between groundwater elevation and river stage.

### 3.2 Groundwater Chemistry

Shallow groundwater chemistry samples were collected from select monitor wells, observation wells, and drive point piezometers during base flow and during the climbing limb, peak, and declining limb of the river and shallow groundwater spring hydrograph. Samples were collected in accordance with Standard Operating

Procedures (CFR SOP). Drive point piezometers had 2 feet of well screen and were installed so that the top of the well screen would be approximately level with the groundwater surface. Both 2-inch and 1 ¼-inch diameter drive points were installed. The analytical results of samples collected from drive point piezometers were for observing trends in groundwater chemistry only, since drive points were not installed using the same protocols as monitor wells, outlined in the CFR SSI SOP.

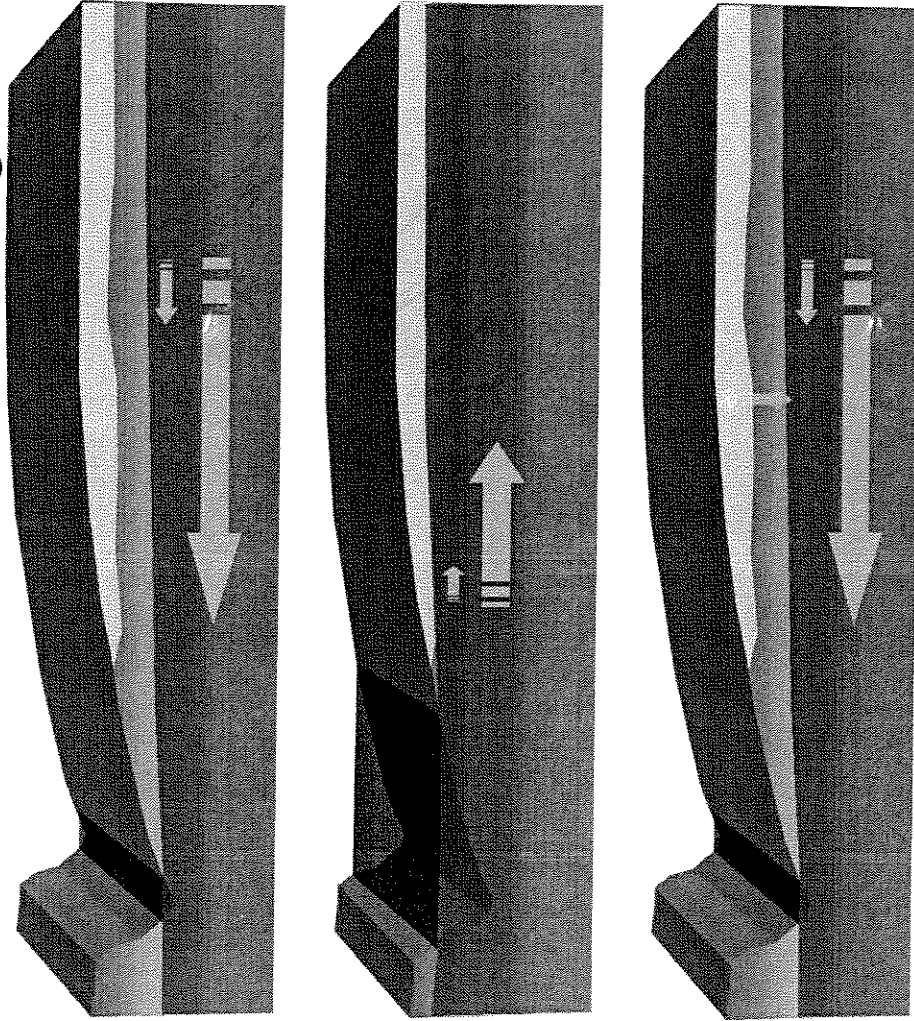
## Clark Fork River Alluvial Groundwater



**Figure 2.2.** Typical cross section through the Clark Fork River floodplain showing variable materials.

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## Clark Fork River-Bank Storage and Metal Transport



**Baseflow** - alluvial groundwater flowing in gravel layer low in metal concentration. Low levels of metals transported in shallow groundwater due to low hydraulic conductivity of upper saturated layer. Metals contained in water recharged through tailings are mostly attenuated.

**Peak flow** - rising groundwater levels force groundwater laterally into banks and upward into metal-enriched tailings. Groundwater flux of metals to river negligible due to reversed gradient.

**Declining limb** - as shallow groundwater system desaturates, water containing metals may flow downward into gravel aquifer and be carried toward the river. Evidence of this transport mechanism would be elevated metal concentrations in shallow groundwater during peak flow and elevated metals in gravel layer during falling limb.

**Figure 2.3. Expected changes in groundwater chemistry during variation in river stage.**

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## **4.0 Results and Discussion**

### **4.1 Hydrostratigraphic Units**

Drill logs from the installation of monitor wells, observation wells, and suction lysimeters were used to describe the hydrostratigraphic units of the shallow groundwater system in the Clark Fork River floodplain. In addition, observations made in soil pits were also utilized.

#### **4.1.1 Control Microwatershed**

Drill logs indicated that the shallow alluvial groundwater system included three distinct hydrologic/soil units (i.e. layers), each with distinct hydrologic and soil properties. The top unit, or layer, is comprised primarily of tailings material. This layer was only partially and seasonally saturated at the control site, with saturated conditions occurring only at bottom of the unit. The control site did not have a well developed buried soil beneath the tailings material, which was dissimilar to most observed soil profiles in the CFR floodplain where tailings are present. The second layer consisted of a sand and gravel mix with fines, and often the top portion of the layer was not saturated. The third layer was dominated by clean heaving sand and in some areas was a gravel and cobble mix.

#### **4.1.2 Treated Microwatershed**

Drill logs from monitor well and suction lysimeter installation indicated three distinct hydrologic/soil units also exist at the treated microwatershed. The top layer consisted of approximately 1 to 3 feet of amended tailings mixed with natural soil through plowing. The bottom portion of the amended tailings and soil was generally only saturated seasonally. Layer 2 and 3 material was similar to that observed at the control site and described in the conceptual hydrologic/soil model. The microwatershed had been revegetated with grasses

The upper layer at the control and treated microwatersheds are similar. However, the materials at the treated site were plowed when lime was incorporated, creating a homogeneous material.

The three layer system observed at the control and treated sites is similar to the description of sediment units presented in Brooks, (1988). Layer 3 (clean heaving sand and Gravel and Cobble mix) is similar to the Brooks gravel and boulder aquifer material and layer 2 (sand and gravel with fines) is similar to the coarse quartz sand with dark fines and gravel described by Brooks. Layer 1 (tailings material) incorporates the top three sediment units outlined by Brooks as tailings, transition unit, and cohesive silt. The transition unit and cohesive silt are discontinuous and thin, where present at the control site. The unit described by Brooks as topsoil is not present at the control site.



## 4.2 Hydraulic Characteristics

The shallow groundwater system in the Clark Fork River floodplain was characterized with a combination of pumping tests, slug tests, and bore hole permeameter tests.

### 4.2.1 Control Microwatershed

Results of bore hole permeameter testing at the control site indicate that the tailings material has a hydraulic conductivity of approximately  $1 \times 10^{-5}$  feet per min (ft/min). The bore hole permeameter tests conducted at the control site are described by ARCO (1997a). The average hydraulic conductivity of layer 2, the sand and gravel with fines, was  $7.5 \times 10^{-3}$  ft/min and was estimated with slug tests using wells OBS-1B, OBS-2B, OBS-3B, and OBS-4B. The observation wells were completed from 2.2 to 4.2 feet below the ground surface. Results of slug tests completed at the control site are described by ARCO (1997b). An analysis of the pumping test results completed at the control site indicates that the hydraulic conductivity of the clean sands and gravel and cobble mix averages approximately  $4 \times 10^{-1}$  ft/min. Additional pumping test analysis and the semi-confined conditions observed suggest that vertical hydraulic conductivity is two to three orders of magnitude less than horizontal hydraulic conductivity (ARCO, 1997a).

### 4.2.2 Treated Microwatershed

Results from tests completed at the treated site indicate that the hydraulic conductivity of the amended tailings material increases below a depth of approximately 12 inches, with values at 4, 12, and 20 inches of  $8.5 \times 10^{-5}$ ,  $6.5 \times 10^{-5}$ , and  $7.1 \times 10^{-4}$  ft/min, respectively. Results of bore hole permeameter tests conducted at the treated site are presented in Appendix A. Slug tests completed in monitor wells MW-1A and MW-1B indicate the average hydraulic conductivity of layer 2, sand and gravel with fines, was  $3.5 \times 10^{-2}$  ft/min. Monitor wells MW-1A and MW-1B were completed between 5 and 10 feet below the ground surface. Data describing the results of slug tests conducted at the treated site are presented in Appendix A.

The amended and revegetated tailings material at the treated microwatershed have somewhat different hydraulic properties than tailings material at the control microwatershed. The amendment process, which mixed the materials, and roots from vegetation appear to slightly increase hydraulic conductivity of the material and create a relatively homogenous material which should have similar vertical and horizontal hydraulic conductivity. The vertical hydraulic conductivity of the fluvially deposited tailings material, at the control site, and the fluvially deposited layer 2 and 3 material at both sites is significantly less than horizontal hydraulic conductivity.

The three-layer hydrologic conceptual model appears to accurately describe the hydrologic characteristics estimated at the control and treated microwatersheds. Overall, data from aquifer testing suggests that hydraulic conductivity increases with depth, and that vertical groundwater flux is not significant when compared to horizontal groundwater flux.

### 4.3 Groundwater Flow and Direction

Water level measurements were collected from monitor wells MW-1A and MW-1B at the treated site and from MW-2A and MW-2B during quarterly sampling since 1994. Figures 4.1 and 4.2 display changes in groundwater elevation and river stage, measured at the United States Geological Survey gauging station located at the Perkins Lane Bridge (Clark fork River at Galen), during the period of record for the treated and control microwatersheds, respectively. The figures represent absolute groundwater elevation, with the control site located downstream and the treated site located upstream of the gauging station.

The figures indicate that the shallow groundwater system in the Clark Fork River floodplain responds quickly to changes in river stage.

In general, groundwater elevations respond quickly to changes in river stage. Groundwater flow at the control and treated microwatersheds is towards the Clark Fork River. The direction of groundwater flow and the general gradient of the shallow groundwater system in the Clark Fork River flood plain do not change significantly when river stage and groundwater elevation change.

In order to more precisely detail changes in the shallow groundwater elevation, in response to changes in the peak stage hydrograph, additional static water level measurements were collected at the control and treated sites during sample collection on 17 May 1997, 31 July 1997, and 21 August 1997. Measurements were also collected on 24 September, 1997 during base river flow conditions at the control site.

#### 4.3.1 Control Microwatershed

Exhibit A displays the potentiometric surface during base flow conditions (24 September 1996) and during the declining limb of the peak hydrograph on 31 July 1997 and 21 August 1997. The figures indicate that groundwater flow direction and gradient do not change significantly as river stage and groundwater elevation changes.

Neutron probe and depth to water measurements collected at the control site indicate that the depth to water is less than 0.5 ft below the ground surface, with the volumetric water content of the tailings material ranging from 30% to 33%, approximately the effective saturation point. The tailings material is capable of moving water vertically through capillary action, creating near saturated material conditions near the surface. Water moves up towards the surface to replace water lost to evaporation, often leaving a salt precipitate at the surface. Changes in water level and soil moisture are displayed in cross section Exhibit C.

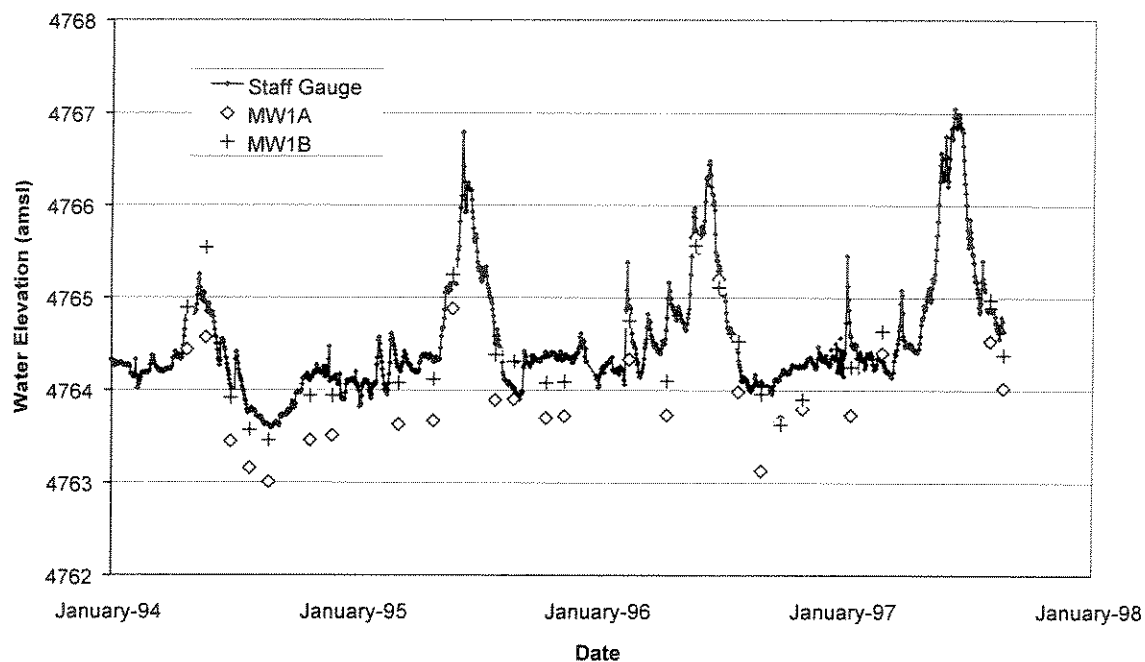
#### 4.3.2 Treated Microwatershed

Exhibit B displays the potentiometric surface during base flow conditions (28 October 1996) and during the declining portion of the hydrograph (31 July 1997 and 21 August 1997). Again, the figures indicate that groundwater flow direction and

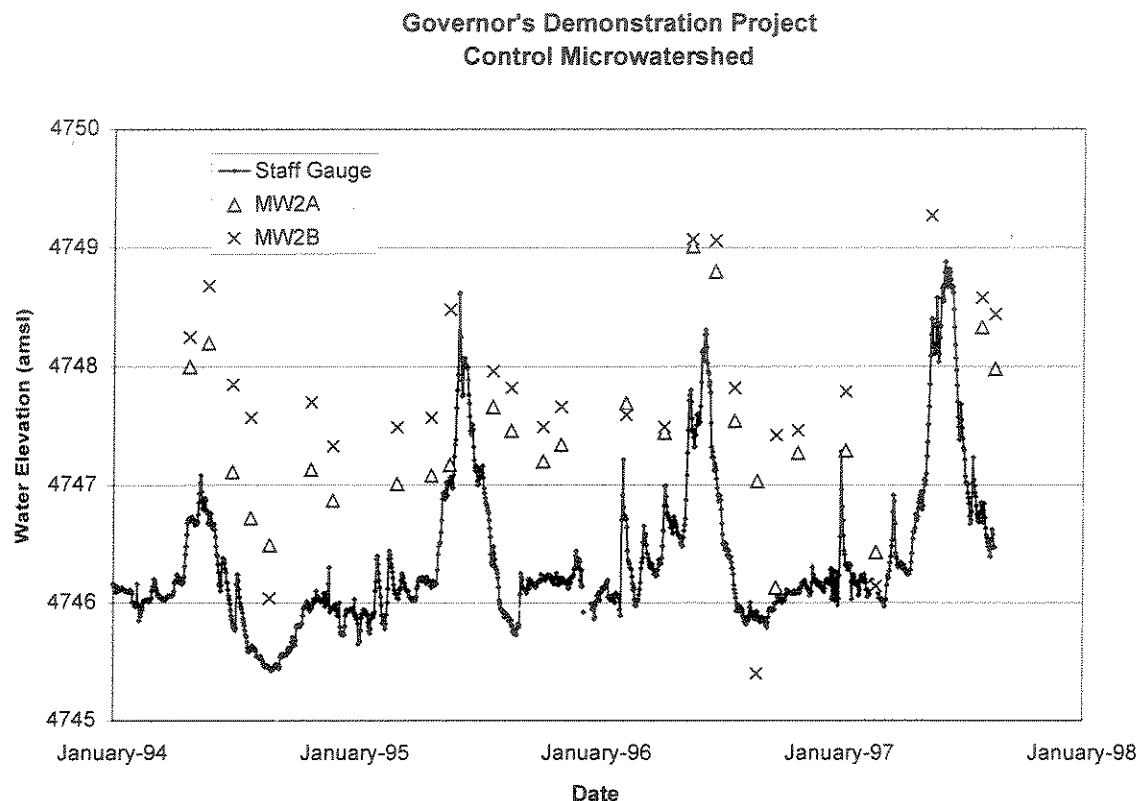
gradient do not change significantly when river stage and groundwater elevation change.

The water level at the treated site is approximately 2 feet below the ground surface on 31 July 1997 and approximately 3 feet below the ground surface on 21 August 1997, in the sand and gravel material with fines. The sand and gravel with fines material does not have the strong capillary action of the tailings material and neutron probe measurements indicate that soil moisture in the amended tailings material does not significantly change and is not at saturated levels. Changes in water level and soil moisture are displayed in cross section, Exhibit D.

**Governor's Demonstration Project  
Treated Microwatershed**



**Figure 4.1. Groundwater elevation at the Governor's Demonstration Project treated microwatershed and river stage at the Clark Fork River near Galen USGS gauging station.**



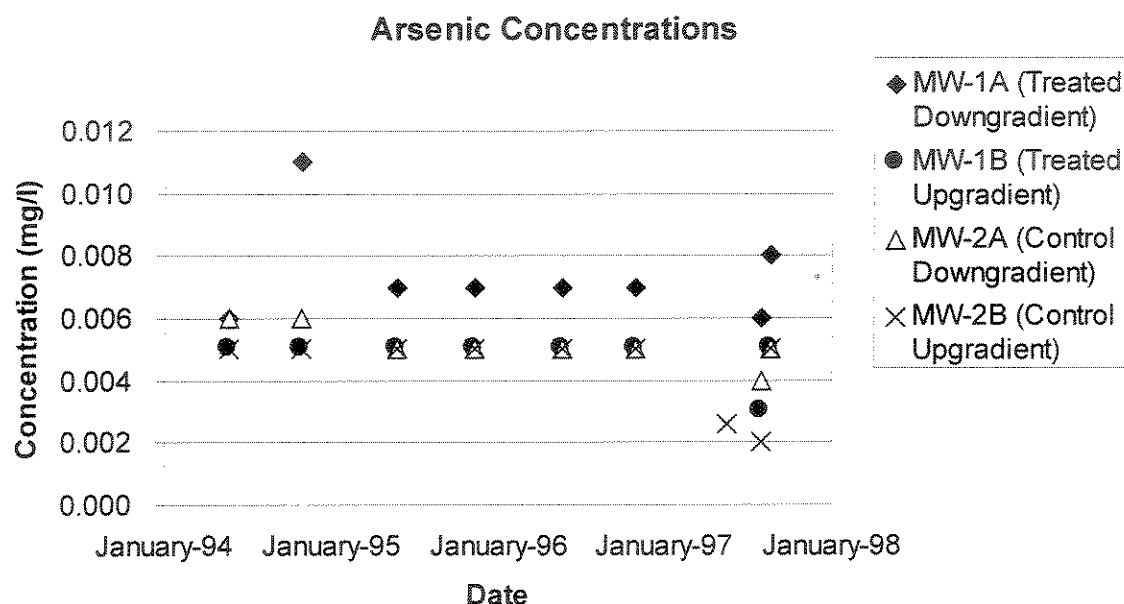
**Figure 4.2. Groundwater elevation at the Governor's Demonstration Project control microwatershed and river stage at the Clark Fork River near Galen USGS gauging station.**

#### 4.4 Groundwater chemistry Characterization

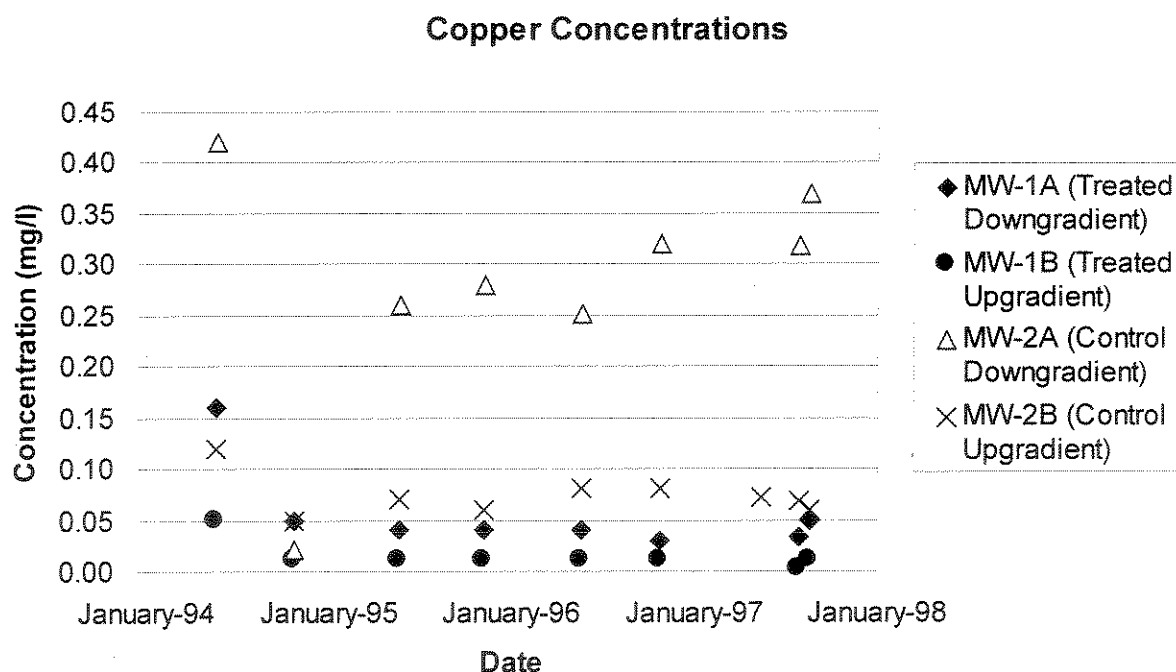
Concentrations of arsenic, copper, and zinc have been monitored at monitor wells MW-1A and MW-1B at the treated microwatershed and MW-2A and MW-2B and the control microwatershed since 1994, with Figures 4.3 through 4.5 displaying changes in concentration. Many values were below detection and a relative change in concentration displayed in the figures were often caused by changes in the detection limit. Recent sampling has also included cadmium and lead. Groundwater chemistry data are presented in Appendix B, with concentrations of constituents of concern presented in Table 4.1. All lead values were below the detection limit for wells MW-1A, MW-1B, MW-2A, and MW-2B and only 2 cadmium values above the detection limit were available, wells MW-1A and MW-2A, and are presented in Table 4.1.

There does not appear to be a significant correlation between the concentrations of constituents of concern and groundwater elevation. Generally, only small changes in chemistry were observed. Downgradient wells had higher copper and zinc than upgradient wells at the control site. At the tested site, zinc and possibly arsenic were higher in downgradient wells.

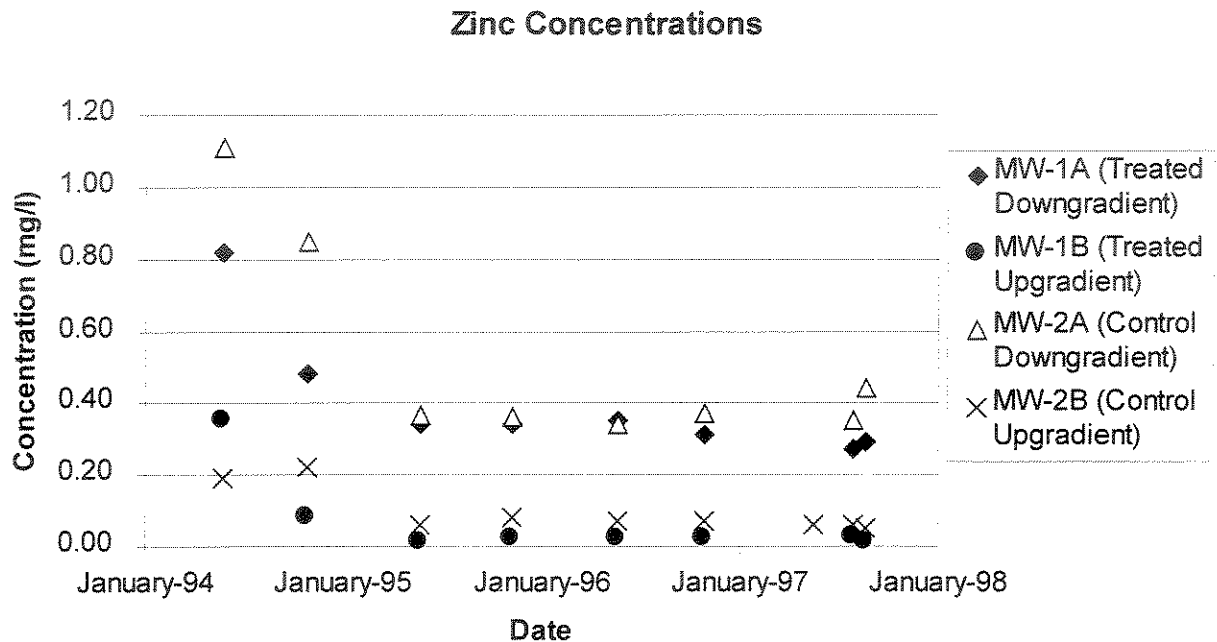
In general, no significant change in concentration was observed with changes in river stage. Arsenic, copper, and zinc values were significantly greater at the control microwatershed at all portions levels of river stage.



**Figure 4.3. Arsenic concentrations at Governor's Demonstration Project treated and control microwatersheds for the period of record.**



**Figure 4.4. Copper concentrations at Governor's Demonstration Project treated and control microwatersheds for the period of record.**



**Figure 4.5. Zinc concentrations at Governor's Demonstration Project treated and control microwatersheds for the period of record.**

**Table 4.1. Concentrations of constituents of concern for monitor wells MW-1A, MW-1B, MW-2A, and MW-2B.**

WELL NUMBER	SAMPLE DATE	As (mg/l)	Cd (mg/l)	Cu (mg/l)	Pb (mg/l)	Zn (mg/l)	
MW-1A	5/23/94	0.006		0.16		0.82	D
MW-1A	10/26/94	0.011		0.05		0.48	U
MW-1A	5/24/95	0.007		0.04		0.34	U
MW-1A	11/8/95	0.007	U	0.04	U	0.34	U
MW-1A	5/21/96	0.007		0.04		0.35	D,U
MW-1A	10/28/96	0.007	U	0.04	U	0.35	D,U
MW-1A	7/31/97	0.006	0.0013	0.034	0.002	B	0.272
MW-1A	8/21/97	0.008	0.001	0.05	0.01	B	0.29
MW-1B	5/23/94	0.005	B	0.05		0.35	
MW-1B	10/26/94	0.005	B	0.01	B	0.08	U
MW-1B	5/24/95	0.005	B	0.01	B	0.01	B,U
MW-1B	11/8/95	0.005	B,U	0.01	B,U	0.02	U
MW-1B	5/21/96	0.005	B	0.01	B	0.02	D,U
MW-1B	10/28/96	0.005	B,U	0.01	B,U	0.02	U
MW-1B	7/31/97	0.003	0.001	B	0.001	B	0.023
MW-1B	8/21/97	0.005	B	0.001	B	0.01	B
MW-2A	5/23/94	0.006		0.42		1.11	
MW-2A	10/26/94	0.006		0.02		0.85	U
MW-2A	5/24/95	0.005	B	0.26		0.37	U
MW-2A	11/8/95	0.005	U	0.28	U	0.36	U
MW-2A	5/21/96	0.005		0.252		0.34	D,U
MW-2A	10/28/96	0.005	B,U	0.32	U	0.37	U
MW-2A	7/31/97	0.004	0.0024	0.318	0.002	B	0.351
MW-2A	8/21/97	0.005	B	0.003	0.37	0.01	B
MW-2B	5/23/94	0.005	B	0.12		0.19	
MW-2B	10/26/94	0.005	B	0.05		0.22	U
MW-2B	5/24/95	0.005	B	0.07		0.06	U
MW-2B	11/8/95	0.005	B,U	0.06	U	0.08	U
MW-2B	5/21/96	0.005	B	0.08		0.07	D,U
MW-2B	5/17/97	0.0026	B	0.003	B	0.072	0.001
MW-2B	10/28/96	0.005	B,U	0.07	U	0.06	U
MW-2B	7/31/97	0.002	0.003	0.069	0.002	B	0.058
MW-2B	8/21/97	0.005	B	0.001	B	0.06	0.01

B = Constituent concentration below detection.

U = Constituent concentration undefendable, spike standard analysis out o range.

D = Relative percent difference > 25% for duplicate sample analysis.

## 5.0 Conclusion

The shallow groundwater system in the Clark Fork River floodplain is dominated by three distinct hydrologic/soil units. The upper unit (layer 1) is primarily tailings, soil, or a mixture of tailings material and soil. The middle layer is primarily sand and gravel with fines, and layer 3 is a clean heaving sand or a gravel and cobble mix.

Analysis of the aquifer test results and the semi-confined conditions observed during aquifer testing suggest that vertical hydraulic conductivity is two to three orders of magnitude less than horizontal hydraulic conductivity, indicating that the flux of water from tailings material to the underlying shallow groundwater system is not significant when compared to horizontal shallow groundwater flux. The hydraulic conductivity increases at deeper depths, with layer 1 having significantly lower hydraulic conductivity than layer 2, and layer 2 having significantly lower hydraulic conductivity than layer 3.

Groundwater elevations vectors, near the river, indicate that groundwater flows downriver (North) with a component of flow towards the Clark Fork River. The shallow groundwater flow direction and gradient does not change significantly when the shallow groundwater elevations change in response to changes in river stage. Groundwater gradients generally reflect the river gradient.

Samples collected from shallow groundwater monitor wells suggest that concentrations of constituents of concern are not affected by changes in river stage and groundwater elevation. Downgradient wells had higher copper and zinc at the control and zinc at the treated microwatershed than did upgradient wells. Concentrations of copper are greater downgradient of the treated site during all portions of the river hydrograph.

Since groundwater gradients and groundwater flux do not increase and since concentrations of constituents of concern do not appear to increase when river stage and groundwater elevation increase during spring peak flow conditions, no significant increase in load is expected, due to changes in river stage. Therefore, load of constituents of concern during high flow conditions is expected to be similar to those estimated during base flow conditions at either the control or treated microwatershed.



## 6.0 References

- ARCO, 1997a. Technical Memo: Aquifer Test Analysis, prepared for ARCO by Schafer and Associates.
- ARCO, 1997b. Technical Memo: Near -Stream Gradients and Slug Tests, prepared for ARCO by Schafer and Associates.
- Brooks, R., 1988. Distribution and Concentration of Metals in Sediments and Water of the Clark Fork River Floodplain, Montana, M.S. Thesis, University of Montana, Missoula, Montana.
- Nimick, D. A., 1990. Stratigraphy and Chemistry of Metal-Contaminated Floodplain Sediments, Upper Clark Fork River Valley, Montana, M.S. Thesis, University of Montana, Missoula, Montana.
- Schafer and Associates, 1991. Final Report on the Clark fork River Demonstration Project, Warm Springs, Montana. Report submitted to the office of the Governor.
- Schafer and Associates, 1997. Clark Fork River Governor's Demonstration Project Monitoring (1993-1996) Report submitted to the office of the Governor.

# **Appendix A**

## **Hydraulic Data**

Bank Full Storage - Governor's Demonstration Project Groundwater Elevation				
Well No	10/28/97	5/17/97	8/1/97	8/21/97
SITE 1 - TREATED MICROWATERSHED				
MW-1A	4763.80	-	4764.53	4764.02
MW-1B	4763.90	-	4764.98	4764.38
DP-1A		-	4764.99	4764.51
DP-1B		-	4764.72	4764.19
DP-1C		-	4764.76	4764.44
Staff Gauge		-	4765.40	4764.90
Well No	9/24/96	5/17/97	8/1/97	8/21/97
SITE 2 - UNTREATED MICROWATERSHED				
MW-2A	4747.00	-	4748.33	4747.98
MW-2B	4747.40	4749.27	-	4748.44
OBS-1A	-	-	-	4749.72
OBS-1B	4746.81	-	4748.22	4747.73
OBS-1C	4746.78	-	4748.24	4747.72
OBS-2A	4747.20	-	-	-
OBS-2B	4747.19	4749.65	4748.48	4748.19
OBS-2C	4747.16	-	4748.45	4748.16
OBS-3A	4747.22	-	-	-
OBS-3B	4747.21	4749.68	4748.49	4748.22
OBS-3C	4747.22	4749.16	4748.49	4748.24
OBS-4A	-	-	-	-
OBS-4B	4747.26	4749.22	4748.49	4748.25
OBS-4C	4747.25	-	-	-
OBS-5	4747.22	-	4748.50	4748.25
OBS-6	4747.22	-	4748.48	4748.24
PW-1	4747.19	-	4748.45	4748.23
DP-1	4746.71	-	-	-
DP-2	4746.71	-	-	-
DP-3	4746.69	-	-	-
DP-4	4747.23	-	4748.44	4748.25
DP-5	-	-	4748.58	4748.25
Staff Gauge	4746.70	-	4747.84	4746.73

## GP FIELD DATA SHEET

SECTION 2: STANDARDIZED PROCEDURE  
FOR PERMEAMETER READINGS  
AND CALCULATIONSDate 7/31/97 Investigator Tiede

Reservoir Constants: (See label on Permeamter)

Combined Reservoirs X	cm <sup>2</sup>
Inner Reservoir Y	cm <sup>2</sup>

☒ CHECK  
RESERVOIR  
USED
Depth of Well Hole 20"

Note: In standardized procedure the radius of the well hole is always 3.0 cm

1st Set of Readings with height of water in well (H<sub>1</sub>) set at 5 cm

READING NUMBER	TIME	TIME INTERVAL (MIN)	WATER LEVEL IN RESERVOIR, (CM)	WATER LEVEL CHANGE, (CM)	RATE OF WATER LEVEL CHANGE, R <sub>1</sub> , (CM/MIN)
	18:13	START			
1	18:15	2	1.10	—	
2	18:17	2	6.00	4.90	2.45
3	18:19	2	11.35	5.35	2.675
4	18:21	2	17.20	5.85	2.925
5	18:23	2	23.30	6.10	3.05
6	18:28	5	38.70	15.40	3.08
7	18:33	5	54.00	15.30	3.06
8	18:38	5	69.40	15.40	3.08
9	18:43	5			

2nd Set of Readings with height of water in well (H<sub>2</sub>) set at 10 cm

READING NUMBER	TIME	TIME INTERVAL (MIN)	WATER LEVEL IN RESERVOIR, (CM)	WATER LEVEL CHANGE, (CM)	RATE OF WATER LEVEL CHANGE, R <sub>2</sub> , (CM/MIN)
	18:50	START			
1	18:52	2	7.30	—	
2	18:54	2	7.70	0.40	0.20
3	18:56	2	8.20	0.50	0.25
4	18:58	2	8.75	0.55	0.275
5	19:00	2	9.30	0.55	0.275
6	19:05	5	10.90	1.60	0.32
7	19:10	5	12.90	2.00	0.40
8	19:15	5	14.90	2.00	0.40
9	19:20	5	16.80	1.90	0.38

RESERVOIR CONSTANT  
2.15

CALCULATIONS

RESERVOIR CONSTANT  
35.64

R, the steady state rate of flow, is achieved when R is the same in three consecutive time intervals.

For the 1st Set of Readings  $\bar{R}_1 = (3.073)/60 = 0.0512$  cm/secFor the 2nd Set of Readings  $\bar{R}_2 = (0.393)/60 = 0.00656$  cm/sec

$$K_{fs} = [(0.041)(35.64)(0.00656)] - [(0.0054)(2.15)(0.0512)] = 3.6 \times 10^{-4} \text{ cm/sec}$$

FIELD SATURATED HYDRAULIC CONDUCTIVITY      RESERVOIR CONSTANT      R<sub>2</sub> - STEADY STATE RATE OF FLOW      RESERVOIR CONSTANT      R<sub>1</sub> - STEADY STATE RATE OF FLOW

$$\phi_m = [(0.0572)(2.15)(0.0512)] - [(0.0237)(35.64)(\quad)] = \quad \text{cm}^2/\text{sec}$$

MATRIC FLUX POTENTIAL      RESERVOIR CONSTANT      R<sub>1</sub> STEADY STATE RATE OF FLOW      RESERVOIR CONSTANT      R<sub>2</sub> STEADY STATE RATE OF FLOW

$$\alpha = (\quad) / (\quad) = \quad \text{cm}^{-1}$$

ALPHA PARAMETER      K<sub>fs</sub>      φ<sub>m</sub>

$$\Delta\theta = (\quad) - (\quad) = \quad \text{cm}^3/\text{cm}^3$$

DELTA THETA      θ<sub>fs</sub>, FIELD SATURATED WATER CONTENT OF SOIL, IN CM / CM      θ<sub>i</sub>, AMBIENT WATER CONTENT OF SOIL, IN CM / CM

$$S = \sqrt{2(\quad)(\quad)} = \quad \text{cm sec}^{-1/2}$$

SORPTIVITY      S<sub>1</sub>      φ<sub>m</sub>

ESTIMATED	CHECK ONE
MEASURED	

## GP FIELD DATA SHEET

SECTION 2: STANDARDIZED PROCEDURE  
FOR PERMEAMETER READINGS  
AND CALCULATIONSDate 07/31/97 Investigator TIEDE

Reservoir Constants: (See label on Permeameter)

Combined Reservoirs X 35.64 cm<sup>2</sup>  
Inner Reservoir Y 2.15 cm<sup>2</sup>☒ CHECK  
RESERVOIR  
USEDDepth of Well Hole 4 inNote: In standardized procedure the radius  
of the well hole is always 3.0 cm1st Set of Readings with height  
of water in well (H<sub>1</sub>) set at 3 cm  
STOP WATCH 15:31 = 08:192nd Set of Readings with height  
of water in well (H<sub>2</sub>) set at 10 cm  
ACTUAL 16:13 = 08:31

READING NUMBER	TIME	TIME INTERVAL (MIN)	WATER LEVEL IN RESERVOIR, (CM)	WATER LEVEL CHANGE, (CM)	RATE OF WATER LEVEL CHANGE, R <sub>1</sub> , (CM/MIN)
	15:31	START			
1	15:33	2	36.20	—	
2	15:35	2	37.48	1.28	0.64
3	15:37	2	38.62	1.14	0.57
4	15:39	2	40.40	1.78	0.89
5	15:41	2	41.86	1.46	0.73
6	15:46	5	45.80	3.94	0.79
7	15:51	5	47.80	4.00	0.80
8	15:56	5	54.00	4.20	0.84
9	15:61	5	58.10	4.10	0.82
10	15:67	5	62.50	4.40	0.88

READING NUMBER	TIME	TIME INTERVAL (MIN)	WATER LEVEL IN RESERVOIR, (CM)	WATER LEVEL CHANGE, (CM)	RATE OF WATER LEVEL CHANGE, R <sub>2</sub> , (CM/MIN)
	16:13	START			
1	16:15	2	07.00		
2	16:17	2	09.30	2.30	1.15
3	16:19	2	12.00	2.70	1.35
4	16:21	2	14.78	2.78	1.39
5	16:23	2	17.35	2.57	1.29
6	16:28	5	23.96	6.55	1.31
7	16:33	5	30.64	6.74	1.35
8	16:38	5	37.53	6.89	1.38
9	16:43	5	44.50	6.97	1.39
10	16:48	5	51.45	6.95	1.39

## CALCULATIONS

the steady state rate of flow, is achieved when R is the same in three consecutive time intervals.

for the 1st Set of Readings  $\bar{R}_1 = \left( \frac{0.83}{R_1} \right) / 60 = 0.0138$  cm/secfor the 2nd Set of Readings  $\bar{R}_2 = \left( \frac{1.39}{R_2} \right) / 60 = 0.0231$  cm/sec

$$K_{fs} = \left[ (0.0041) \left( \frac{2.15}{\text{RESERVOIR CONSTANT}} \right) \left( \frac{0.0231}{\bar{R}_2 \text{ - STEADY STATE RATE OF FLOW}} \right) \right] - \left[ (0.0054) \left( \frac{2.15}{\text{RESERVOIR CONSTANT}} \right) \left( \frac{0.0138}{\bar{R}_1 \text{ - STEADY STATE RATE OF FLOW}} \right) \right] = 4.34 \times 10^{-5} \text{ cm/sec}$$

$$\phi_m = \left[ (0.0572) \left( \frac{2.15}{\text{RESERVOIR CONSTANT}} \right) \left( \frac{0.0138}{\bar{R}_1 \text{ - STEADY STATE RATE OF FLOW}} \right) \right] - \left[ (0.0237) \left( \frac{2.15}{\text{RESERVOIR CONSTANT}} \right) \left( \frac{0.0231}{\bar{R}_2 \text{ - STEADY STATE RATE OF FLOW}} \right) \right] = 5.20 \times 10^{-4} \text{ cm}^2/\text{sec}$$

$$\alpha = \frac{(4.34 \times 10^{-5})}{(5.20 \times 10^{-4})} = 0.0835 \text{ cm}^{-1}$$

$$\Delta\theta = \left( \frac{\theta_s}{\theta_a} \right) - \left( \frac{\theta_s}{\theta_a} \right) = \text{cm}^3/\text{cm}^3$$

DELTA THETA  $\theta_s$ , FIELD SATURATED  
WATER CONTENT OF SOIL, IN CM / CM  $\theta_a$ , AMBIENT WATER CONTENT  
OF SOIL, IN CM / CM

$$S = \sqrt{2 \left( \frac{\theta_s}{\theta_a} \right) \left( \frac{\theta_s}{\theta_a} \right)} = \text{cm sec}^{-1/2}$$

SORPTIVITY

ESTIMATED  
MEASUREDCHECK  
ONE

# GP FIELD DATA SHEET

## SECTION 2: STANDARDIZED PROCEDURE FOR PERMEAMETER READINGS AND CALCULATIONS

Date 7/31/97 Investigator TIERE

Reservoir Constants: (See label on Permeamter)

Combined Reservoirs X for 10cm cm<sup>2</sup>  
Inner Reservoir Y for 5cm cm<sup>2</sup>

☐ CHECK  
RESERVOIR  
USED  
☒

Depth of Well Hole 12 in

Note: In standardized procedure the radius of the well hole is always 3.0 cm

1st Set of Readings with height of water in well (H<sub>1</sub>) set at 5 cm

2nd Set of Readings with height of water in well (H<sub>2</sub>) set at 10 cm

READING NUMBER	TIME	TIME INTERVAL (MIN)	WATER LEVEL IN RESERVOIR, (CM)	WATER LEVEL CHANGE, (CM)	RATE OF WATER LEVEL CHANGE, R <sub>1</sub> , (CM/MIN)
	16:55	START			
1	16:57	2	5.30	—	
2	16:59	2	10.10	4.80	2.40
3	17:01	2	15.10	5.00	2.50
4	17:03	2	20.63	5.53	2.765
5	17:05	2	26.00	5.37	2.685
6	17:10	5	39.30	13.30	2.66
7	17:15	5	52.90	13.60	2.72
8	17:20	5	66.55	13.65	2.73
9	17:25	5			

CELL CONSTANT 2.15

READING NUMBER	TIME	TIME INTERVAL (MIN)	WATER LEVEL IN RESERVOIR, (CM)	WATER LEVEL CHANGE, (CM)	RATE OF WATER LEVEL CHANGE, R <sub>2</sub> , (CM/MIN)
	17:30	START			
1	17:32	2	10.40	—	
2	17:34	2	10.90	0.50	0.25
3	17:36	2	11.30	0.40	0.20
4	17:38	2	11.80	0.50	0.25
5	17:40	2	12.20	0.40	0.20
6	17:45	5	13.40	1.20	0.24
7	17:50	5	14.60	1.20	0.24
8	17:55	5	15.70	1.10	0.22
9	18:00	5	16.80	1.10	0.22

CELL CONSTANT 35.64

### CALCULATIONS

the steady state rate of flow, is achieved when R is the same in three consecutive time intervals.

for the 1st Set of Readings  $\bar{R}_1 = (2.725) / 60 = 0.0454$  cm/sec

for the 2nd Set of Readings  $\bar{R}_2 = (0.23) / 60 = 0.00383$  cm/sec

$$K_{fs} = [(0.0041)(35.64)(0.00383)] - [(0.0054)(2.15)(0.0454)] = 3.256 \times 10^{-5} \text{ cm/sec}$$

FIELD SATURATED HYDRAULIC CONDUCTIVITY      RESERVOIR CONSTANT      R<sub>2</sub> - STEADY STATE RATE OF FLOW      RESERVOIR CONSTANT      R<sub>1</sub> - STEADY STATE RATE OF FLOW

$$\phi_m = [(0.0572)(2.15)(0.0454)] - [(0.0237)(35.64)(0.00383)] = 2.348 \times 10^{-3} \text{ cm}^2/\text{sec}$$

MATRIC FLUX POTENTIAL      RESERVOIR CONSTANT      R<sub>1</sub> - STEADY STATE RATE OF FLOW      RESERVOIR CONSTANT      R<sub>2</sub> - STEADY STATE RATE OF FLOW

$$\alpha = (3.256 \times 10^{-5}) / (2.348 \times 10^{-3}) = 0.0139 \text{ cm}^{-1}$$

PHI PARAMETER      K<sub>fs</sub>      φ<sub>m</sub>

$$\Delta\theta = ( ) - ( ) = \text{cm}^3/\text{cm}^3$$

DELTA THETA      θ<sub>fs</sub>, FIELD SATURATED WATER CONTENT OF SOIL, IN CM/CM      θ<sub>i</sub>, AMBIENT WATER CONTENT OF SOIL, IN CM/CM

$$S = \sqrt{2( ) ( )} = \text{cm sec}^{-1/2}$$

SORPTIVITY      S<sub>11</sub>      φ<sub>m</sub>

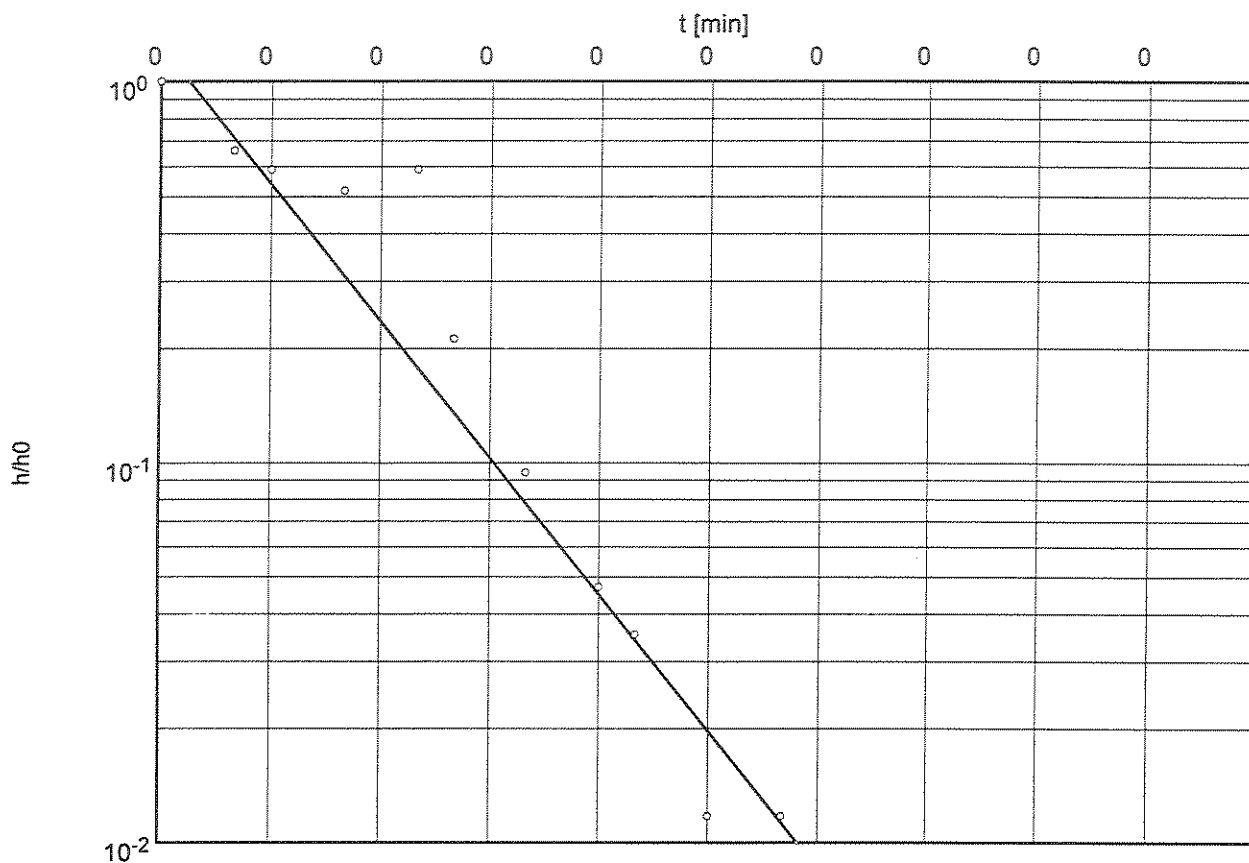
ESTIMATED  
MEASURED

CHECK  
ONE

Slug Test No. 1

Test conducted on: 7/31/97

MW-1A



Well 1A - Test 1

Hydraulic conductivity [ft/min]:  $5.72 \times 10^{-2}$

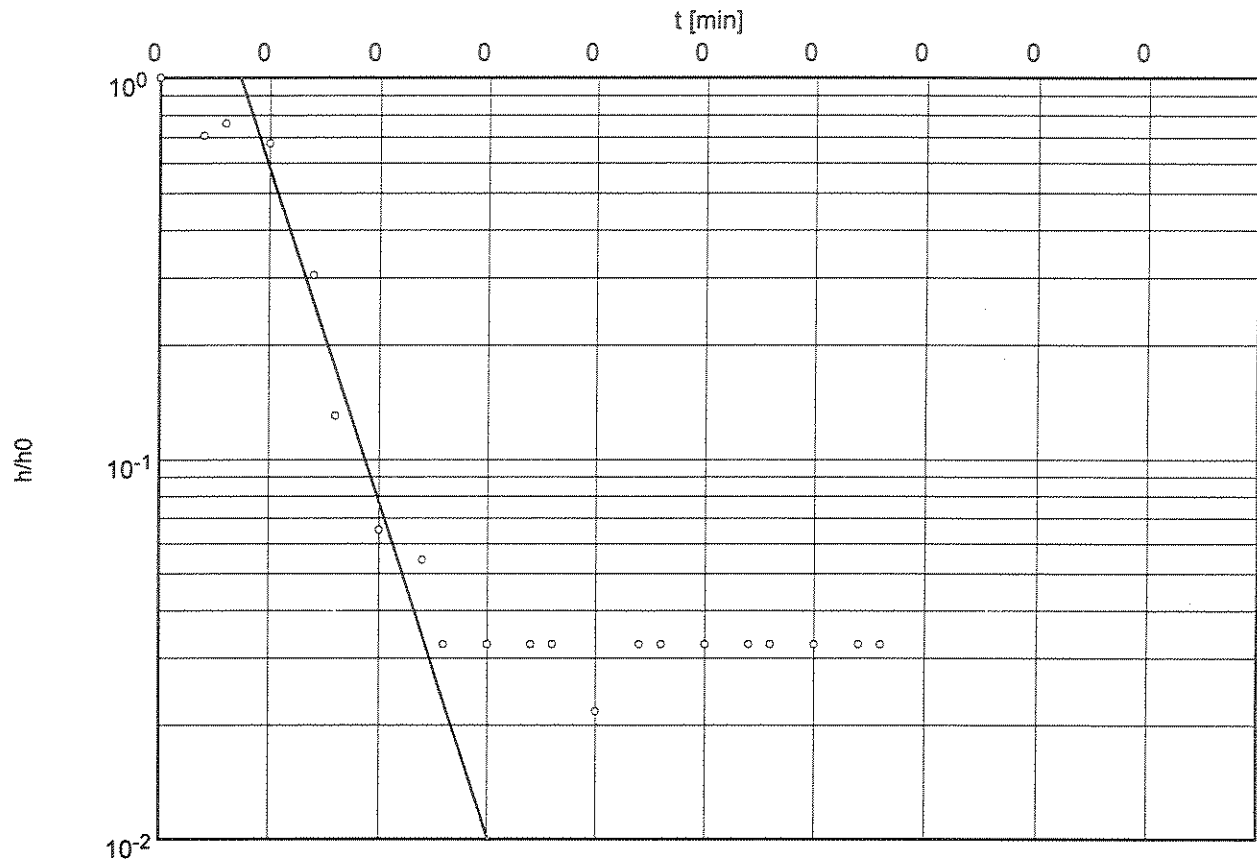
[illegible]



Slug Test No. 2

Test conducted on: 7/31/97

MW-1A



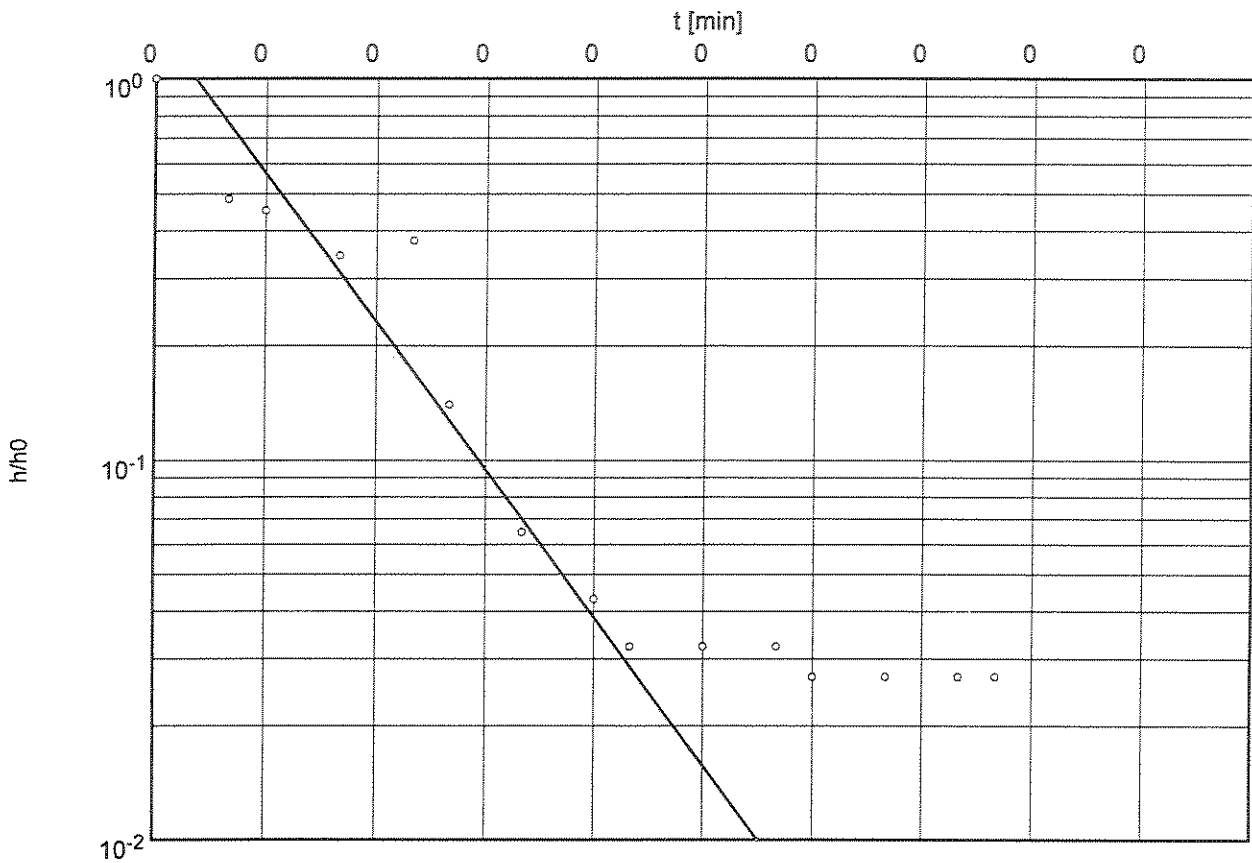
Hydraulic conductivity [ft/min]:  $8.38 \times 10^{-2}$

[illegible]

Slug Test No. 3

Test conducted on: 7/31/97

MW-1A



Well 1A - Test 3

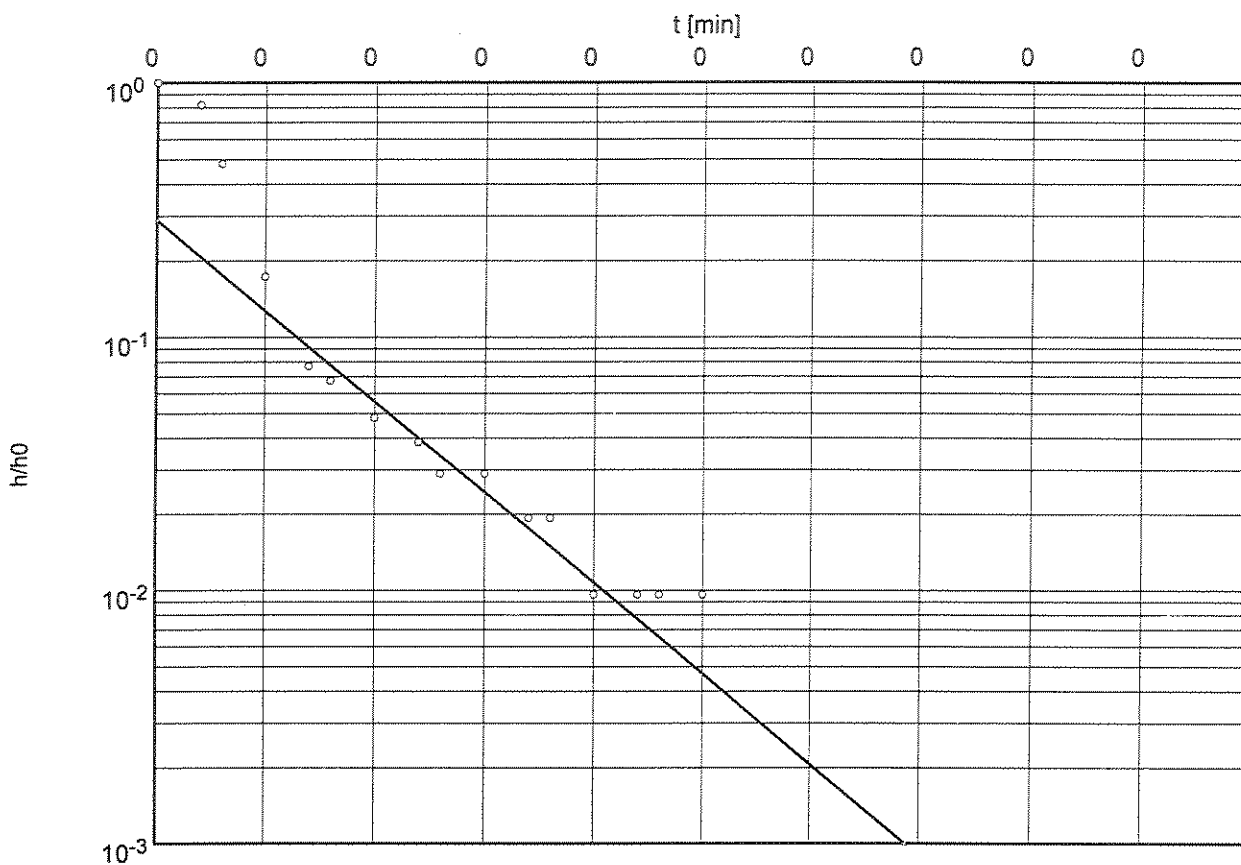
Hydraulic conductivity [ft/min]:  $6.19 \times 10^{-2}$

[illegible]

Slug Test No. 1

Test conducted on: 7/31/97

MW-2B



Well 1B - Test 1

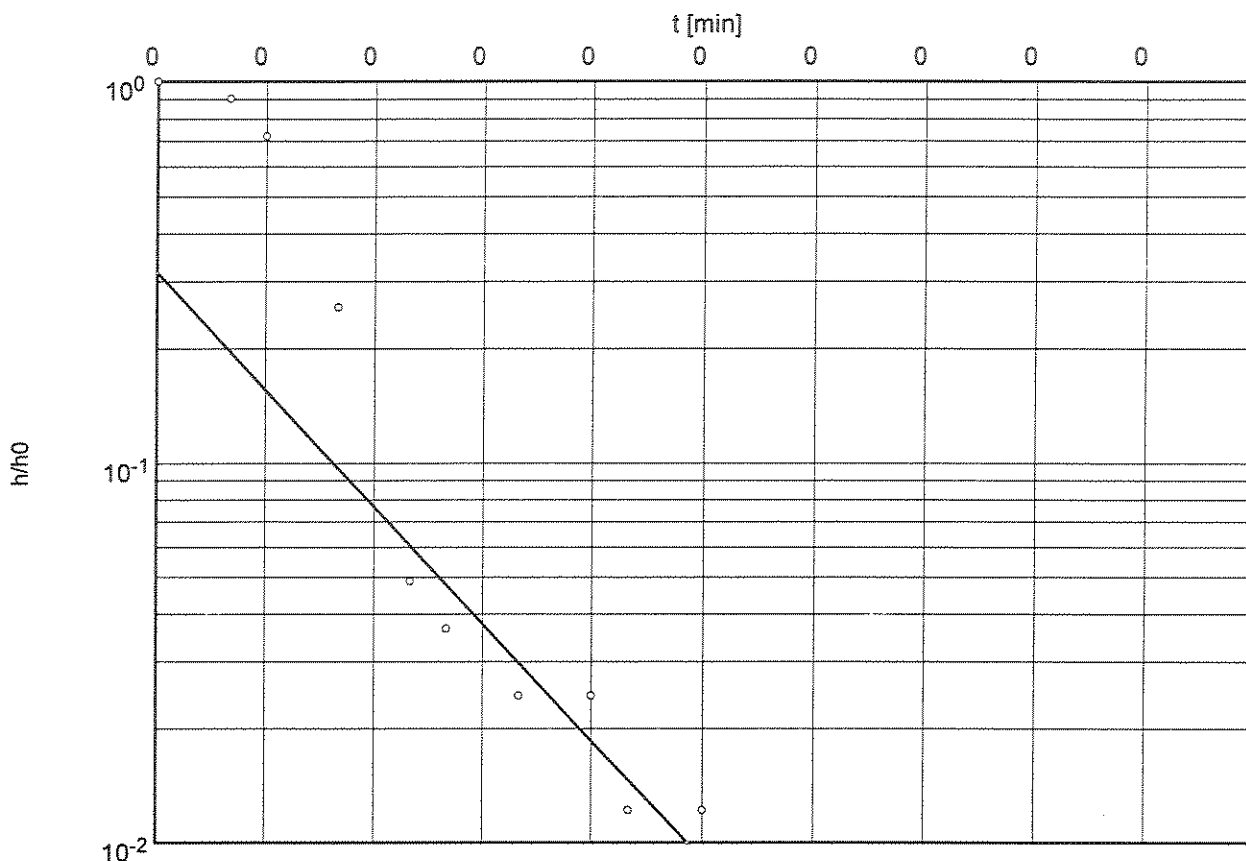
Hydraulic conductivity [ft/min]:  $3.41 \times 10^{-2}$

[illegible]

Slug Test No. 2

Test conducted on: 7/31/97

MW-1B



o Well 1B - Test 2

Hydraulic conductivity [ft/min]:  $4.90 \times 10^{-2}$

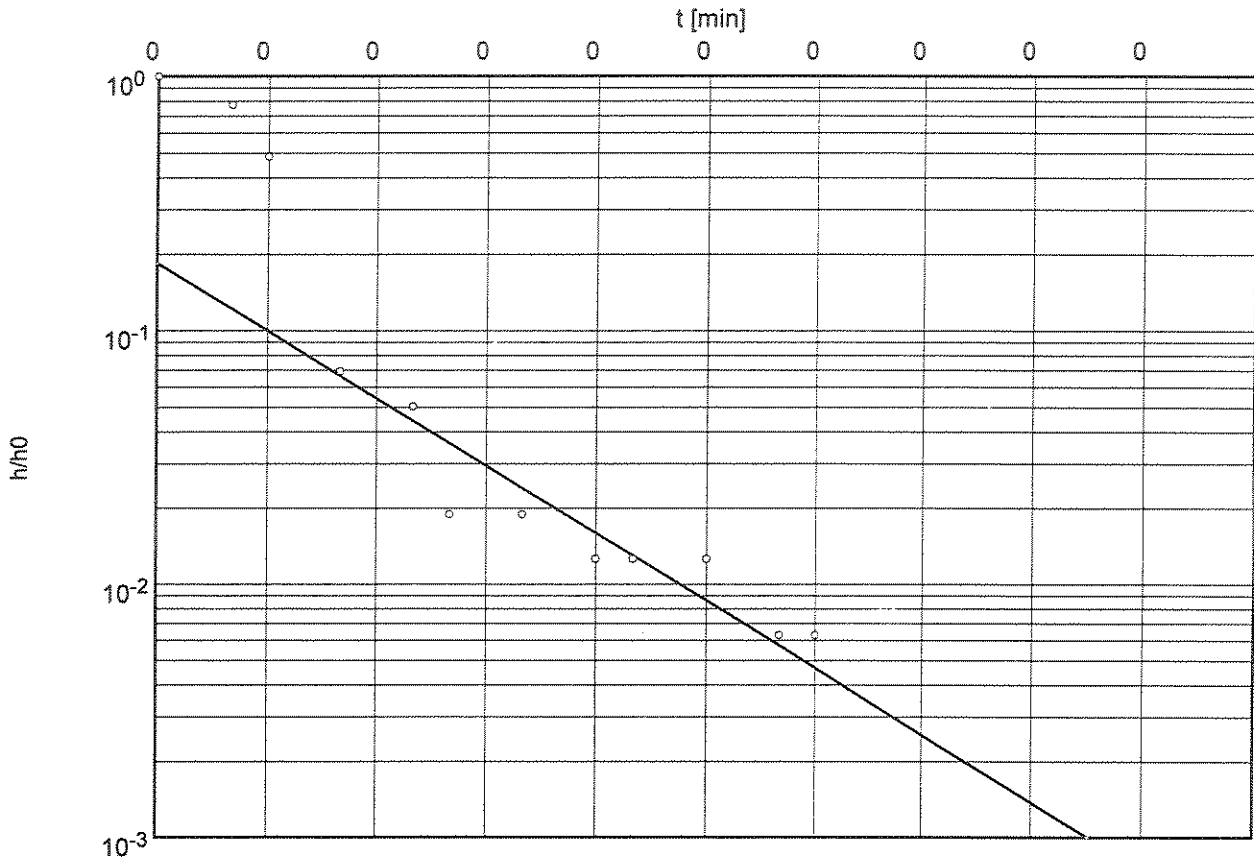
[illegible]



Slug Test No. 3

Test conducted on: 7/31/97

MW-2B



Well 1B - Test 3

Hydraulic conductivity [ft/min]:  $4.23 \times 10^{-2}$

[illegible]

**Appendix B-1**  
**Control Chemical Data**

[illegible]

[illegible]

Governor's Demonstration Project			
Treated Site Groundwater Chemistry			
Monitor Well MW-1A			
Metal	5/17/97	7/31/97	8/21/97
Arsenic	-	0.006	0.008
Cadmium	-	0.0013	0.001
Copper	-	0.034	0.05
Lead	-	0.002 B	0.01 B
Zinc	-	0.272	0.29
pH		7.00	6.37
EC		688	728
Monitor Well MW-1B			
Metal	5/17/97	7/31/97	8/21/97
Arsenic	-	0.003	0.005 B
Cadmium	-	0.0001 B	0.001 B
Copper	-	0.001 B	0.01 B
Lead	-	0.002 B	0.01 B
Zinc	-	0.023	0.01 B
pH		7.16	6.29
EC		542	590
Drive Point DP-1A			
Metal	5/17/97	7/31/97	8/21/97
Arsenic	-	0.007	0.018
Cadmium	-	0.0019	0.001
Copper	-	0.035	0.01
Lead	-	0.002 B	0.01 B
Zinc	-	0.532	0.24
pH		6.47	6.07
EC		410	602
Drive Point DP-1B			
Metal	5/17/97	7/31/97	8/21/97
Arsenic	-	0.001	0.005 B
Cadmium	-	0.0032	0.004
Copper	-	0.121	0.02
Lead	-	0.002	0.01 B
Zinc	-	0.71	0.4
pH		6.55	6.47
EC		525	389
Drive Point DP-1C			
Metal	5/17/97	7/31/97	8/21/97
Arsenic	-	0.089	0.055
Cadmium	-	0.0209	0.009
Copper	-	0.307	0.1
Lead	-	0.004	0.01 B
Zinc	-	3.33	1.51
pH		5.87	5.85
EC		588	510

Notes:

All results in mg/l

B - Element concentration below detection

Bank Full Storage - Governor's Demonstration Project		
Major Ions Chemistry		
Treated Microwatershed		
	MW1B	MW1A
Constituent	7/31/97	8/21/97
Potassium	10	14
Sodium	37	42
Calcium	93	120
Magnesium	16	21
Sulfate	155	248
Chloride	11	15
Carbonate as CO <sub>3</sub>	0	0
Bicarbonate as HCO <sub>3</sub>	231	220
Alkalintiy	190	180
Fluoride	1.1	1.15
Nitrate plus Nitrite as N	0.05 B	0.05 B
Phosphorus	0.01	0.01 B
Barium, dissolved	0.041	0.1 B
Iron, dissolved	0.37	0.07

Notes:

All results in mg/l

B - Element concentration below detection

**Appendix B-2**  
**Treated Chemical Data**



[illegible]

CLARK FORK GOVERNORS PROJECT FOR CONTROL AND VEGETATED MICROWATERSHEDS  
MONITORING WELL SAMPLING RESULTS (Organized by Well Number)

WELL SAMPLE		FIELD PARAMETERS					LABORATORY ANALYSIS																				
		No.	DATE	SWL (bgs)	pH	EC (uhos/cm)	(C)	MAJOR IONS mg/l										DISSOLVED METALS mg/l									
								K	Na	Ca	Mg	SO4	Cl	CO3	HCO3	F	NO3	Alk	P	As	Cu	Zn					
MW-2B	26-Apr-95			2.42	7.04	528	5.6																				
MW-2B	24-May-95			1.51	7.32	467	4.9	4	16	92	14	159	7	0	160	0.74	0.05	B	131	0.42	D	0.005	B	0.07		0.06	U
MW-2B	28-Jul-95			2.03	7.34	549	11.4																				
MW-2B	25-Aug-95			2.17	6.69	568	15.1																				
MW-2B	12-Oct-95			2.50	6.67	578	10.2																				
MW-2B	08-Nov-95			2.33	5.93	530	10.1	4	15	88	14	144	6	0	168	0.84	0.36	U	138	0.19	D	0.005	B,U	0.06	U	0.08	U,D
MW-2B	13-Feb-96			2.4	7.14	528	5																				
MW-2B	10-Apr-96			2.5	7.21	440	4.5																				
MW-2B	21-May-96			0.92	7.04	512	11.4	4	16	95	15	154	8	0	173	0.84	0.05	B	142	0.05		0.005	B,U	0.08	U	0.07	D,U
MW-2B	25-Jun-96			0.93	6.78	558	11.6																				
MW-2B	25-Jul-96			2.17	6.69	553	13.5																				
MW-2B	28-Aug-96			4.59	6.85	594	10.9																				
MW-2B	26-Sep-96			2.57	6.34	491	10.5																				
MW-2B	28-Oct-96			2.53	6.67	421	7.3	4	17	78	13	131	7	0	172	0.87	U	0.44	141	0.48	D	0.005	B,U	0.07	U	0.06	U

B - Element concentration below detection.

U - Element concentration undefendable, spike standard analysis out of range.

D - Relative Percent Difference >25% for duplicate sample analysis.

u:\147\data\96\data\mw\l.xls

Governor's Demonstration Project				
Control Site Groundwater Chemistry				
Monitor Well MW-2A				
Metal	5/17/97	7/31/97	8/21/97	
Arsenic	-	0.004	0.005 B	
Cadmium	-	0.0024	0.003	
Copper	-	0.318	0.37	
Lead	-	0.002 B	0.01 B	
Zinc	-	0.351	0.44	
pH	-	6.96	5.80	
EC	-	468	578	
Monitor Well MW-2B				
Metal	5/17/97	7/31/97	8/21/97	
Arsenic	0.0026 B	0.002	0.005 B	
Cadmium	0.003 B	0.0003	0.001 B	
Copper	0.072	0.069	0.06	
Lead	0.001 U	0.002 B	0.01 B	
Zinc	0.0597	0.058	0.05	
pH	7.22	7.49	6.04	
EC	200	415	443	
Observation Well OBS-1B				
Metal	9/24/96	5/17/97	7/31/97	8/21/97
Arsenic	0.144	-		0.1
Cadmium	0.001 B	-		0.001 B
Copper	0.01 B	-		0.01 B
Lead	0.01 B	-		0.01 B
Zinc	0.44	-		0.27
pH	6.95	-		5.87
EC	776	-		583
Observation Well OBS-1C				
Metal	9/24/96	5/17/97	7/31/97	8/21/97
Arsenic	0.013	-	0.013	
Cadmium	0.001 B	-	0.0006	
Copper	0.01 B	-	0.01	
Lead	0.01 B	-	0.002 B	
Zinc	0.17	-	0.11	
pH	7.08	-	7.13	
EC	675	-	462	

Governor's Demonstration Project					
Control Site Groundwater Chemistry					
Observation Well OBS-2B					
Metal	9/24/96	5/17/97	7/31/97	8/21/97	
Arsenic	0.007	0.0046	B		0.014
Cadmium	0.003	0.0065			0.006
Copper	0.19	0.402			0.18
Lead	0.01 B	0.001	U		0.01 B
Zinc	0.45	1.37			1.23
pH	6.91	6.7			5.58
EC	545	656			484
Observation Well OBS-2C					
Metal	9/24/96	5/17/97	7/31/97	8/21/97	
Arsenic	0.019		0.024		
Cadmium	0.001		0.0008		
Copper	0.08		0.024		
Lead	0.01 B		0.002 B		
Zinc	0.25		0.058		
pH	7.01		7.45		
EC	565		445		
Observation Well OBS-3B					
Metal	9/24/96	5/17/97	7/31/97	8/21/97	
Arsenic	0.006	0.0083	B	-	-
Cadmium	0.001 B	0.0044	B	-	-
Copper	0.18	0.193		-	-
Lead	0.01 B	0.001	U	-	-
Zinc	0.31	0.341		-	-
pH	6.93	6.82		-	-
EC	556	400		-	-
Observation Well OBS-3C					
Metal	9/24/96	5/17/97	7/31/97	8/21/97	
Arsenic	0.006	0.0034	B	-	-
Cadmium	0.001 B	0.0019	B	-	-
Copper	0.03	0.0452		-	-
Lead	0.01 B	0.001	U	-	-
Zinc	0.11	0.0901		-	-
pH	7.01	6.92			
EC	526	550			